GENERAL STRUCTURAL NOTES

I. GENERAL

A ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE SECTIONS OF THE CALIFORNIA BUILDING CODE (CBC) 2013 EDITION, AND ALL OTHER PUBLICATIONS LISTED.

B ALL DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, ALL CONTRACT DOCUMENTS AND SPECIFICATIONS.

C DETAILS SHOWN ON THE STRUCTURAL DRAWINGS ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS.

D DIMENSIONS SHOWN SHALL TAKE PRECEDENCE OVER SCALE ON THE PLANS, SECTIONS AND DETAILS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT IMMEDIATELY.

E NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.

F CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON ROOF OR FLOOR FRAMING MEMBERS. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD.

G REVIEW OF THE SHOP DRAWINGS BY THE ARCHITECT/ENGINEER IS FOR GENERAL COMPLIANCE WITH THE INFORMATION GIVEN IN THE CONTRACT DOCUMENTS. THE CONTRACTOR WILL REMAIN RESPONSIBLE FOR ALL ERRORS OF DETAILING, FABRICATION AND FOR THE CORRECT FITTING OF ALL STRUCTURAL MEMBERS INCLUDING COORDINATION OF ALL TRADES.

H ALL DETAIL CHANGES DESIRED SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER IN WRITING. SHOP DRAWING SUBMITTALS DO NOT CONSTITUTE CHANGES IN WRITING UNLESS ACCOMPANIED BY A LETTER INDICATING ALL PROPOSED CHANGES. ALL SUCH CHANGES SHALL BE CLEARLY IDENTIFIED ON THE SHOP DRAWINGS.

I ALL WORK IS TO BE PERFORMED IN COMPLIANCE WITH THESE CONSTRUCTION DOCUMENTS. ANY DISCREPANCIES ARE TO BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND THE ENGINEER IMMEDIATELY. REQUIRED REVISED DESIGNS TO CORRECT DEFICIENCIES OR DEVIATIONS SHALL BE PREPARED BY THE DESIGN ARCHITECT OR ENGINEER FOR REVIEW AND APPROVAL BY THE BUILDING INSPECTION DEPARTMENT AT THE CONTRACTOR'S EXPENSE. ALL CORRECTIVE WORK IS TO BE COMPLETED IN ACCORDANCE WITH THE APPROVED REVISED CONSTRUCTION DOCUMENTS.

II. SITE PREPARATION

A FOUNDATION DESIGN: SOIL BEARING PRESSURE: 1500 PSF FOR FOUNDATIONS IN NATIVE SOIL.

III. CONCRETE

A GENERAL: ALL CONCRETE WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF THE ACI MANUAL OF CONCRETE PRACTICE AND THE CBC.

B REINFORCING MATERIAL:

1 DEFORMED ASTM A615, GRADE 40 (#4 BARS AND SMALLER)
GRADE 60 (#5 BARS AND LARGER)

2 WELDED WIRE FABRIC, ASTM A185 – PROVIDE FROM SHEETS ONLY – ROLLED FABRIC PROHIBITED.

C CONCRETE STRENGTH:

PSI @ 28 DAYS	MAX AGG	MAX SLUMP
SLAB 3000	3/4"	4"
FTGS 3000	1 1/2"	4"

SPECIAL INSPECTION OF CONCRETE IS NOT REQUIRED – 3000 PSI CONCRETE IS SPECIFIED AND REQUIRED HOWEVER ALL CONCRETE DESIGNS ARE BASED ON 2500 PSI (MAX) – CBC SECTION 1704.4 EXCEPTIONS 1 & 2

D ADMIXTURES: ONLY AS APPROVED BY THE ENGINEER.

E WELDING OF INSERTS: ALL WELDING OF REINFORCING STEEL SHALL BE WITH LOW-HYDROGEN ELECTRODES IN ACCORDANCE WITH THE LATEST RECOMMENDATIONS OF THE AMERICAN WELDING SOCIETY (A.W.S D1.4–05).

F LAP SPLICES: 48 BAR DIAMETERS FOR GRADE 60 REINFORCING OR
32 BAR DIAMETERS FOR GRADE 40 REINFORCING.

G COVER TO BARS: UNLESS OTHERWISE SHOWN OR NOTED, COVER TO REINFORCING BARS SHALL BE AS STATED PER CBC SECTION 1907.7.1

H CONCRETE CURING: KEEP CONCRETE CONTINOUSLY WET FOR 7 DAYS (MEMBRANE CURING) AS APPROVED BY THE ENGINEER.

I FORM REMOVAL: SIDE OF FOOTINGS AND SLABS ON GRADE, MINIMUM 2 DAYS.

J VIBRATION: VIBRATE ALL CONCRETE IN PLACE WITH A MECHANICAL VIBRATOR USED BY EXPERIENCED PERSONNEL.

K TESTING: IN ACCORDANCE WITH ACI 318–08, CHAPTER 16.

IV. STRUCTURAL STEEL AND MISCELLANEOUS METALS

A GENERAL: ALL WELDING OF STRUCTURAL STEEL SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF THE "WELDING CODE" (A.W.S D1.1–08).

B MATERIALS:

1 STRUCTURAL STEEL – ASTM A36

2 STRUCTURAL TUBING – ASTM A500, GRADE B

3 STEEL PIPE – ASTM A53

4 BASE PLATES AND CONNECTION PLATES – A36

5 NUTS AND BOLTS – ASTM A307

6 WELDING ROD – HEAVILY COATED, CONFORMING WITH A.W.S. "SPECIFICATION FOR ARC WELDING ELECTRODES" OF CLASSIFICATION NUMBERS SUITABLE TO THE WORK TO BE DONE. USE LOW-HYDROGEN ELECTRODES FOR WELDING OF REINFORCING BARS.

C STEEL STUDS AND JOISTS:

1 GRADES: ASTM 446 GRADE D (50 KSI),
ASTM 448 GRADE A (33 KSI)

2 GALVANIZED COATING FOR STUDS, TRACK AND JOISTS ARE TO BE ASTM A653 OR ASTM A525.

3 SCREWS: #10 WAFER HEAD SCREWS (USE LENGTH APPROPRIATE TO TASK).

D OPEN WEB STEEL JOISTS (TRUSSES)

1 OPEN WEB STEEL JOISTS ARE TO BE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH SJI'S "RECOMMENDED CODE OF STANDARD PRACTICE OF STEEL JOISTS AND JOIST GIRDERS".

2 SEE DRAWINGS FOR LOADING CRITERIA.

3 DESIGN OF OPEN WEB JOISTS SHALL BE UNDER THE DIRECT SUPERVISION OF A CALIFORNIA REGISTERED CIVIL ENGINEER. SEALED AND WET SIGNED DESIGN CALCULATIONS AND DRAWINGS SHALL HAVE OWNER'S REVIEW PRIOR TO START OF MANUFACTURE.

4 MANUFACTURER AND ERECTOR ARE RESPONSIBLE FOR BRIDGING, TEMPORARY BRACING, AND COORDINATION OF THE JOISTS, JOIST GIRDERS, AND BRIDGING CONNECTIONS.

E SHOP DRAWINGS: SUBMIT FOR ALL STRUCTURAL STEEL AND MISCELLANEOUS IRON.

V. STRUCTURAL WOOD

A MATERIALS:

1 FOUNDATION SILLS, NAILERS, AND LEDGERS IN DIRECT CONTACT WITH CONCRETE OR MASONRY AND WITHIN 6 FEET OF THE GROUND: PRESERVATIVE TREATED DF #1 OR BETTER.

2 ALL 4x AND 6x POSTS AND COLUMNS: DF #1 OR BETTER.

3 ALL OTHER 6x MEMBERS: DF #1 OR BETTER

4 ALL OTHER 2x AND 4x FRAMING MEMBERS: DF #2 OR BETTER.

5 ALL 2x, 4x, AND 6x MEMBERS SHALL HAVE A MAXIMUM MOISTURE CONTENT OF 19% AT TIME OF "IN SERVICE USE".

6 PLYWOOD: EACH PANEL SHALL BE IDENTIFIED WITH THE GRADE TRADEMARK OF THE APA. NOTE: THE PRESSURE TREATED SILL PLATES ARE CORROSIVE THEREFORE ALL PLYWOOD NAILING INTO THE PRESSURE TREATED SILL PLATES IS TO BE HOT DIPPED GALVANIZED.

WALL PLY: 3/8" CD–EXT. GLUE 24/0
FLOOR PLY: 3/4" CD–EXT. GLUE 48/24
ROOF PLY: 15/32" CD–EXT. GLUE 32/16

OPT: O.S.B. – SEE G/S1.2

B PARALLAM BEAMS:

1 ALL PARALLAM 2.0E PSL BEAMS SHALL BE I.C.C. APPROVED AND BE FABRICATED BY "i LEVEL BY WEYERHAEUSER" CORPORATION.

2 STRESSES: BENDING – Fb = 2900 PSI
MODULUS OF ELASTICITY – E = 2000 KSI
COMPRESSION PERPENDICULAR TO GRAIN – Fc = 650 PSI
HORIZONTAL SHEAR – Fv = 290 PSI

C GLUED-LAMINATED BEAMS:

1 COMBINATION 24F–V4 (UNLESS NOTED OTHERWISE): ALL LAMINATIONS ARE TO BE DF MATERIAL.

2 STRESSES: BENDING (24F–V4) – Fb = 2400 PSI IN BOTTOM FACE & 1850 PSI IN TOP FACE; BENDING (24F–V8) – Fb = 2400 PSI IN BOTH FACES; COMPRESSION PERPENDICULAR TO GRAIN – Fc = 650 PSI.

3 APPEARANCE: INDUSTRIAL GRADE (ARCHITECTURAL GRADE WHERE EXPOSED).

4 FABRICATION: IN STRICT ACCORDANCE WITH CBC SECTION 2303.1 AND AITC 117–04. MANUFACTURING AND DESIGN ARE TO ASSURE THAT QUALITY MATERIALS AND WORKMANSHIP HAVE BEEN USED, FURNISH AN AITC. CERTIFICATE OF CONFORMANCE TO THE ENGINEER AND THE BUILDING DEPARTMENT FOR EACH LOT OF MEMBERS FABRICATED AND DELIVERED TOGETHER.

5 STANDARD CAMBER IS BASED ON A 1600 FOOT RADIUS

D PRESERVATIVE TREATMENT: TREAT ALL WOOD BELOW A HEIGHT OF 6 FEET THAT IS IN DIRECT CONTACT WITH OR EMBEDDED IN CONCRETE OR MASONRY, INCLUDING SILLS, NAILERS, LEDGERS, WOOD GROUNDS, BLOCKING AND OTHER WOOD, WITH CHEMONITE, OR APPROVED EQUAL, IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND CBC SECTION 2303.1 (TREAT ENDS OF PRESSURE TREATED SILLS WHERE SILLS ARE CUT). PRESSURE TREATED LUMBER IS CORROSIVE TO STEEL PRODUCTS. ALL NAILING, BOLTING AND STEEL PLATING THAT ARE IN DIRECT CONTACT WITH THE PRESSURE TREATED SILL PLATE ARE TO BE HOT DIPPED GALVANIZED. ALL "SIMPSON" METAL FASTENERS AND CONNECTORS COMING IN CONTACT WITH THE PRESSURE TREATED SILL PLATE ARE TO BE "Z MAX" GALVANIZED BY THE MANUFACTURER. (OPTION: GALVANIZED NAILING AND CONNECTORS ARE NOT REQUIRED IF SILL PLATES WITH "DISODIUM OCTABORATE TERRAHYDRATE (DOT)" PRESSURE TREATING CHEMICALS ARE USED – COORDINATE WITH THE PROJECT STRUCTURAL ENGINEER IF "DOT" TREATED SILL PLATES ARE TO BE USED).

E MACHINE BOLTS AND LAG BOLTS:

1 BOLTS AND NUTS: ASTM A307

2 WASHERS: A STANDARD CUT WASHER SHALL BE PROVIDED AT EACH BOLT HEAD AND NUT PLACED NEXT TO WOOD. PROVIDE MALLEABLE IRON WASHERS (MIW) OR STEEL PLATE WASHERS WHERE INDICATED ON THE DRAWINGS.

3 BOLT HOLES: MAXIMUM 1/16" LARGER THAN BOLTS ACCURATELY LOCATED. OVERSIZED OR SLOTTED HOLES ARE NOT PERMITTED UNLESS SPECIFICALLY NOTED ON THE DRAWINGS.

4 LAG BOLTS: LEAD HOLES FOR THE THREADED PORTION SHALL BE 75% OF THE SHANK DIAMETER WITH A DEPTH EQUAL TO THE LENGTH OF THE BOLT, AND LEAD HOLE FOR THE UNTHREADED PORTION SHALL BE EQUAL TO THE DIAMETER AND LENGTH OF THE SHANK.

5 ALL BOLTS COMING IN CONTACT WITH THE PRESSURE TREATED SILL PLATES ARE TO BE HOT DIPPED GALVANIZED.

F NAILED JOINTS: COMMON WIRE NAILS OR SPIKES. FOR MINIMUM REQUIREMENTS REFER TO THE TYPICAL NAILING SCHEDULE. ALL NAILS COMING IN CONTACT WITH THE PRESSURE TREATED PLATES OR OTHER PRESSURE TREATED MEMBERS ARE TO BE HOT DIPPED GALVANIZED.

G MISC. METAL CONNECTORS: ALL SHEET METAL CONNECTORS USED FOR CONNECTING STRUCTURAL WOOD MEMBERS SHALL HAVE CBC AND ICC APPROVAL AND BE 'SIMPSON STRONG-TIE' CONNECTORS OR EQUAL APPROVED BY THE ENGINEER. ALL CONNECTORS SHALL BE GALVANIZED OR PROVIDED WITH APPROVED CORROSION PROTECTIVE PAINT. ALL CONNECTORS OR FASTENERS USED THAT ARE TO BE IN CONTACT WITH THE PRESSURE TREATED PLATES OR OTHER PRESSURE TREATED MEMBERS ARE TO BE "Z MAX" GALVANIZED.

VI. TYPICAL NAILING SCHEDULE

UNLESS OTHERWISE SHOWN OR NOTED, THE GENERAL NAILING REQUIREMENTS SHALL COMPLY WITH CBC CHAPTER 23 AND THE FOLLOWING ADDITIONAL MINIMUM REQUIREMENTS. THE MORE STRINGENT REQUIREMENTS SHALL TAKE PRECEDENCE. ALL NAILS USED TO ATTACH TO THE PRESSURE TREATED PLATES ARE TO BE HOT DIPPED GALVANIZED.

TYPICALLY USE 16d NAILS FOR CONNECTING 2x MEMBERS AND 8d OR 10d NAILS FOR CONNECTING 1x MEMBERS WITH A MINIMUM OF 2 NAILS AT EACH JOINT.

SUB-BORE WHEN NAILS TEND TO SPLIT WOOD AND SUB-BORE FOR 20d AND LARGER NAILS (THE DRILL DIAMETER SHALL BE 0.75 x NAIL DIAMETER).

STUDS AND POSTS: (FULL HEIGHT BETWEEN SILL AND TOP PLATE) STUDS TO

SILL OR TOP PLATE .... 2–16d END NAIL OR 2–10d TOE-NAILS EACH SIDE

4x POST TO SILL ...."SIMPSON" A34 EA SIDE EACH END

6x POST TO SILL ...."SIMPSON" A35 EA SIDE EACH END

ADJACENT STUDS AND POSTS..........16d @ 16"

STUDWALL BLOCKING....................2–16d END NAIL OR 2–10d TOE-NAIL EACH END EACH SIDE

DOUBLE TOP PLATE: (MINIMUM 6'–0" LAPS AT SPLICE)

PLATE TO PLATE ....16d @ 16"

AT ALL INTERSECTIONS....................3–16d AT PLATE LAP

AT TYPICAL SPLICES....................ADDITIONAL 10–16d EACH SIDE OF SPLICES UNLESS NOTED OTHERWISE (REFER TO FRAMING PLANS & DETAILS)

JOIST TO TOP PLATE OR TRUSS TO BEAM....................2–16d TOE-NAIL EACH SIDE (PROVIDE FRAMING CLIPS WHERE INDICATED IN DRAWINGS)

JOIST BLOCKING: (MIN 2x BLKG FULL DEPTH OF JOIST; TYPICAL @ ALL SUPPORTS AND AS INDICATED ON DRAWINGS)

BLOCKING TO JOIST....................2–16d END NAIL OR 2–10d TOE-NAIL EACH SIDE

BLOCKING TO PLATE....................3–16d TOE-NAIL (UNLESS NOTED OTHERWISE – REFER TO THE FRAMING DETAILS)

HEADERS AND BEAMS (UNLESS OTHERWISE SPECIFIED IN THE DRAWINGS):

TO TOP OF POST...................."SIMPSON" AC SERIES POST CAPS

TO SIDE OF POST...................."SIMPSON" HH SERIES

COMMON WIRE NAIL SCHEDULE:

PENNYWEIGHT	NAIL LENGTH	NAIL DIAMETER
8d	2 1/2"	0.131"
10d	3"	0.148"
12d	3 1/4"	0.148"
16d	3 1/2"	0.162"
20d	4"	0.192"

VII. PREMANUFACTURED SECOND FLOOR TRUSSES

A MATERIALS: TRUSSES ARE TO BE "I" SOLID WEB TRUSSES WITH STRUCTURAL WOOD CHORDS AND OSB WEBS BY RED BUILT SEE THE ICC-ES EVALUATION REPORT 2994.

B DESIGN: "I" TRUSSES ARE AN "OFF THE SHELF" PREMANUFACTURED ENGINEERED WOOD PRODUCT. THE TRUSS CHORDS AND WEBS ARE DESIGNED AND THE TRUSSES CONSTRUCTED TO PROVIDE THE ALLOWABLE DESIGN STRESSES FOR THESE TRUSSES.

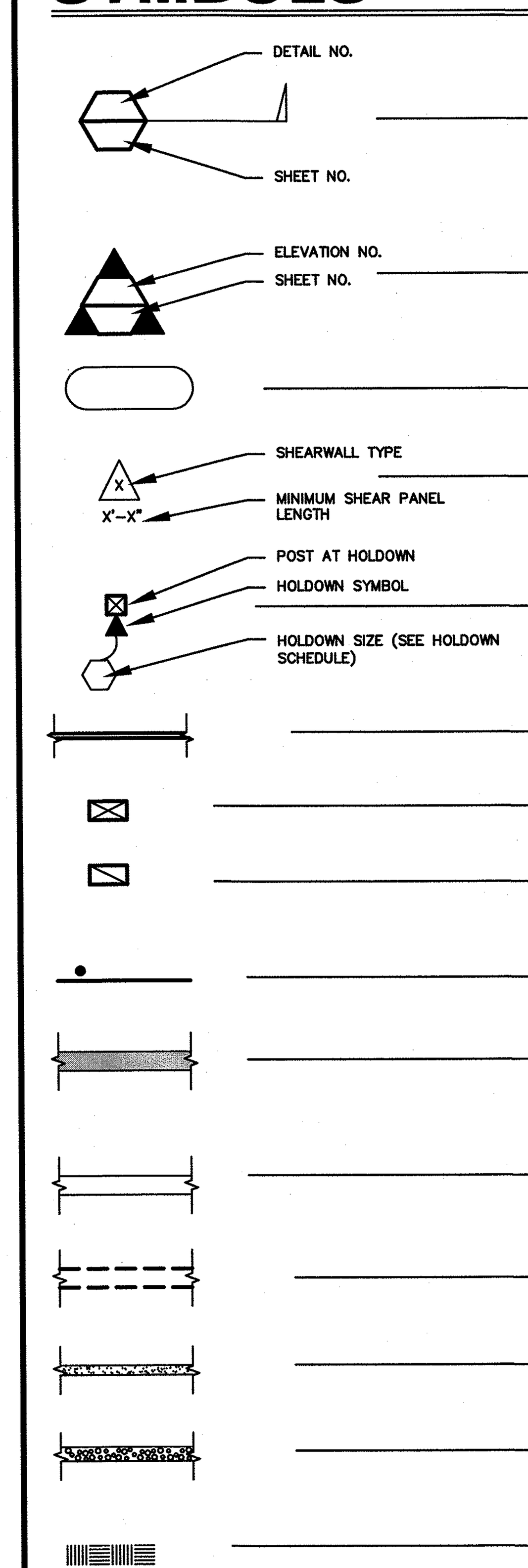
C FOR THE DESIGN LOADS REFER TO THE FLOOR FRAMING NOTES (PART OF THE STRUCTURAL NOTES).

NOTE 1 THE TRUSSES ARE TO BE DESIGNED TO CONFORM WITH THE DETAILS ON THE DRWINGS AND THE MINIMUM DESIGN LOADS INDICATED ABOVE.

ABBREVIATIONS

ADJ	ADJACENT
ALT	ALTERNATE
A BOLT(S)	ANCHOR BOLT(S)
ARCH DRWGS	ARCHITECTURAL DRAWINGS
@	AT
BM	BEAM
BRG	BEARING
BLK	BLOCK
BLKG	BLOCKING
BOT	BOTTOM
BRDG	BRIDGING
CBC	CALIFORNIA BUILDING CODE
CL	CENTER LINE
CTRD	CENTERED
CLR	CLEAR
COL	COLUMN
CONC	CONCRETE
CMU	CONCRETE MASONRY UNIT
CONSTR	CONSTRUCTION
CONT	CONTINUOUS
CTRSK	COUNTERSINK
DIAG	DIAGONAL
DIA	DIAMETER
DBL	DOUBLE
DF	DOUGLAS FIR (LARCH)
DRWGS	DRAWINGS
EA	EACH
FTG	FOOTING
FNDN	FOUNDATION
FLR	FLOOR
FRAM	FRAMING
GLB	GLUED-LAMINATED BEAM
HORIZ	HORIZONTAL
ICC	INTERNATIONAL CODE COUNCIL
MAX	MAXIMUM
MIN	MINIMUM
N/A	NOT APPLICABLE
NTS	NOT TO SCALE
OPT	OPTIONAL
PERPEN	PERPENDICULAR
PL	PLATE
PLY	PLYWOOD
PREMAN	PREMANUFACTURED
PRESS TRTD	PRESSURE TREATED
REINF	REINFORCING
REQD	REQUIRED
SCHED	SCHEDULE
SHT	SHEET
SIM	SIMILAR
SQ	SQUARE
STD	STANDARD
STL	STEEL
TOEN	TOE-NAIL
T & B	TOP & BOTTOM
TPI	TRUSS PLATE INSTITUTE
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W/	WITH

SYMBOLS



DETAIL REFERENCE

FRAMING ELEVATION REFERENCE

RELATIVE ELEVATION DESIGNATION

TYPICAL PLYWOOD SHEAR PANEL DESIGNATION – SEE G/S1.2

TYPICAL HOLDOWN DESIGNATION – SEE H/S1.2

PLYWOOD (SECTION)

WOOD MEMBER (CONTINUOUS) IN SECTION

WOOD MEMBER (BLOCKING) IN SECTION

REINFORCING BARS IN SECTION

NEW STRUCTURAL WALLS (WOOD STUD OR STEEL STUD) IN PLAN

EXISTING STRUCTURAL WALLS (WOOD STUD OR STEEL STUD) IN PLAN

EXISTING STRUCTURAL WALLS TO BE REMOVED IN PLAN

SAND IN SECTION

GRAVEL IN SECTION

EARTH IN SECTION

FLOOR FRAMING NOTES

1 DESIGN FLOOR LOADS:

FLOOR
DL –––– 20.0 PSF
LL –––– 80.0 PSF

2 THE TRUSS MANUFACTURER IS TO VERIFY TRUSS DEPTHS AND DETAILING FOR ADEQUACY AND NOTIFY THE ARCHITECT / ENGINEER IMMEDIATELY OF ANY DISCREPENCIES.

4 FOR SPECIAL INSPECTION REQUIREMENTS REFER TO THE GENERAL STRUCTURAL NOTES AND ANY ADDITIONAL REQUIREMENTS PER THE BUILDING INSPECTION DEPARTMENT,

5 REFER TO ADDITIONAL PREMANUFACTURED TRUSS NOTES SHEET S1.1.

6 ALL TRUSSES ARE TO BE FABRICATED AT AN ICC APPROVED FABRICATION FACILITY.

7 PROVIDE TOP PLATE SPLICE PER B1/S1.2 TYP UNLESS SHOWN OR NOTED OTHERWISE.

8 DRILLING OF HOLES, DRIVING OF HEAVY SCREWS OR USE OF LAG BOLTS IN THE BOTTOM CHORDS OF THE FLOOR TRUSSES IS NOT PERMITTED. DO NOT CUT OR DRILL FLOOR TRUSSES.

9 DAMAGED FLOOR TRUSSES ARE NOT TO BE USED.

10 FOR ALL NON-BEARING INTERIOR WALLS TO THE FLOOR TRUSSES – SEE C/S1.2

FOUNDATION NOTES

1 THE SOILS/PROJECT ENGINEER IS TO OBSERVE AND APPROVE THE FOUNDATION EXCAVATIONS AND REINFORCING PRIOR TO PLACEMENT OF THE CONCRETE.

2 ANCHOR BOLTS FOR ALL HOLDOWNS AND STEEL COLUMNS ARE TO BE WIRED IN PLACE OR HELD IN PLACE BY A TEMPLATE PRIOR TO THE PLACEMENT OF THE CONCRETE, THEY ARE NOT TO BE ADDED WHILE THE CONCRETE IS WET.

3 FOR SPECIAL INSPECTION REQUIREMENTS REFER TO THE GENERAL STRUCTURAL NOTES AND ANY ADDITIONAL REQUIREMENTS PER THE BUILDING INSPECTION DEPARTMENT.

4 FOR ALL SLAB DEPRESSIONS, SLOPED SLABS, FLOOR DRAINS, SPECIAL EQUIPMENT OR INSERTS – SEE THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND DRAWINGS OF OTHER TRADES.

DESIGN CRITERIA

This set of plans MUST be kept on the job at all times and unlawful to make any changes or alterations on same with written permission from Building Safety Division. City of Mode
REVIEWED FOR CODE COMPLIANCE
DEC 30 2014
By______ Permit No.______
The stamping of this plan SHALL NOT be held to permit or to be approval of the violation of any provisions of any Ordinance or L
Occupancy of structure not permitted until after final appro

WIND DESIGN DATA

BASIC WIND SPEED: 110 MPH

WIND EXPOSURE: EXPOSURE "C"

RISK CATEGORY: II

EARTHQUAKE DESIGN DATA

SEISMIC IMPORTANCE FACTOR: I_e = 1.0

SHORT TERM SPECTRAL RESPONSE: S_s = 0920 S_{DS} = 0.694

1 SECOND SPECTRAL RESPONSE: S_1 = 0.337 S_{D1} = 0.388

SITE CLASS: "D"

SEISMIC DESIGN CATEGORY: "D"

BASIC SEISMIC FORCE RESISTING SYSTEM: LIGHT FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE (ASCE 7–10 TABLE 12.2–1, ITEM A.15)

SEISMIC RESPONSE COEFFICIENT: C_s = 0.107

RESPONSE MODIFICATION FACTOR: R = 6.5

DESIGN BASE SHEAR: C_s x EFFECTIVE SEISMIC WEIGHT OF THE BUILDING

ANALYSIS DESIGN PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

SEISMIC DESIGN CATEGORY

THE STRUCTURE IS IN SEISMIC DESIGN CATEGORY "D" PER ASCE 7–10 TABLE 11.6–1

WIND DESIGN

THE STRUCTURE IS IN AN AREA WITH WIND EXPOSURE CATEGORY "C" AND A BASIC WIND SPEED OF 110 MPH.

12-12-14

L Street Architects Limited
1414 L Street
Modesto, California 95354
209-575-1415 209-575-4374 fax
www.lstreetarchitects.com

PELTON WYLIE ENGINEERING, INC
2813 COFFEE ROAD SUITE D1
MODESTO, CALIFORNIA 95355
TEL: (209)575-9619 FAX: (209)575-0274

PROPOSED TENANT IMPROVEMENT FOR:
CENTER FOR HUMAN SERVICES
2000 W BRIGGSMORE, SUITE I
MODESTO, CALIFORNIA 95350

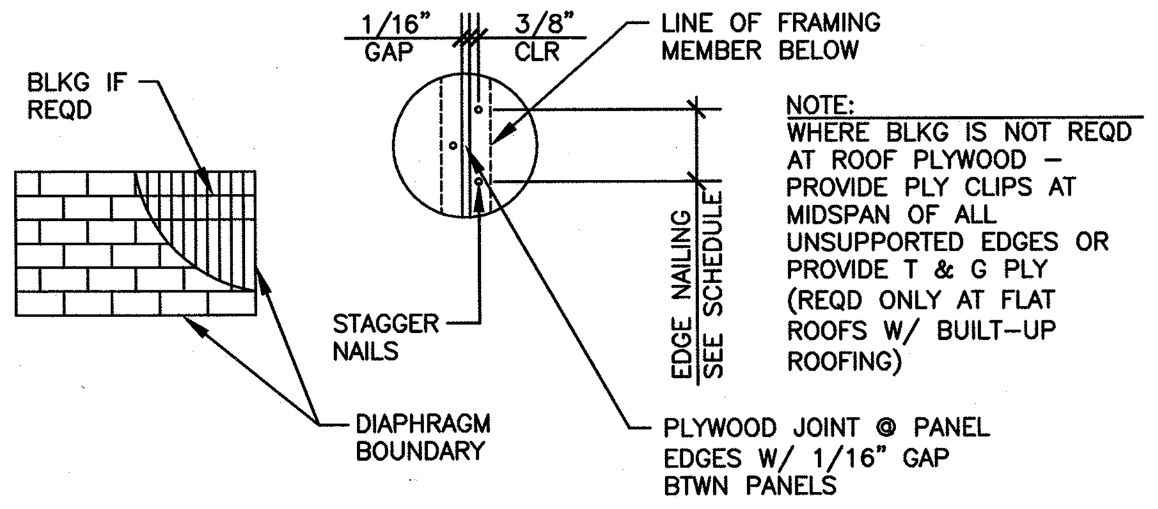
REVISIONS:

DRAWN: SG
DATE: OCT 2014
FILE:
JOB: LSA 015-14
PRINT DATE:

STRUCTURAL NOTES

S1.1

A N/A GENERAL STRUCTURAL NOTES

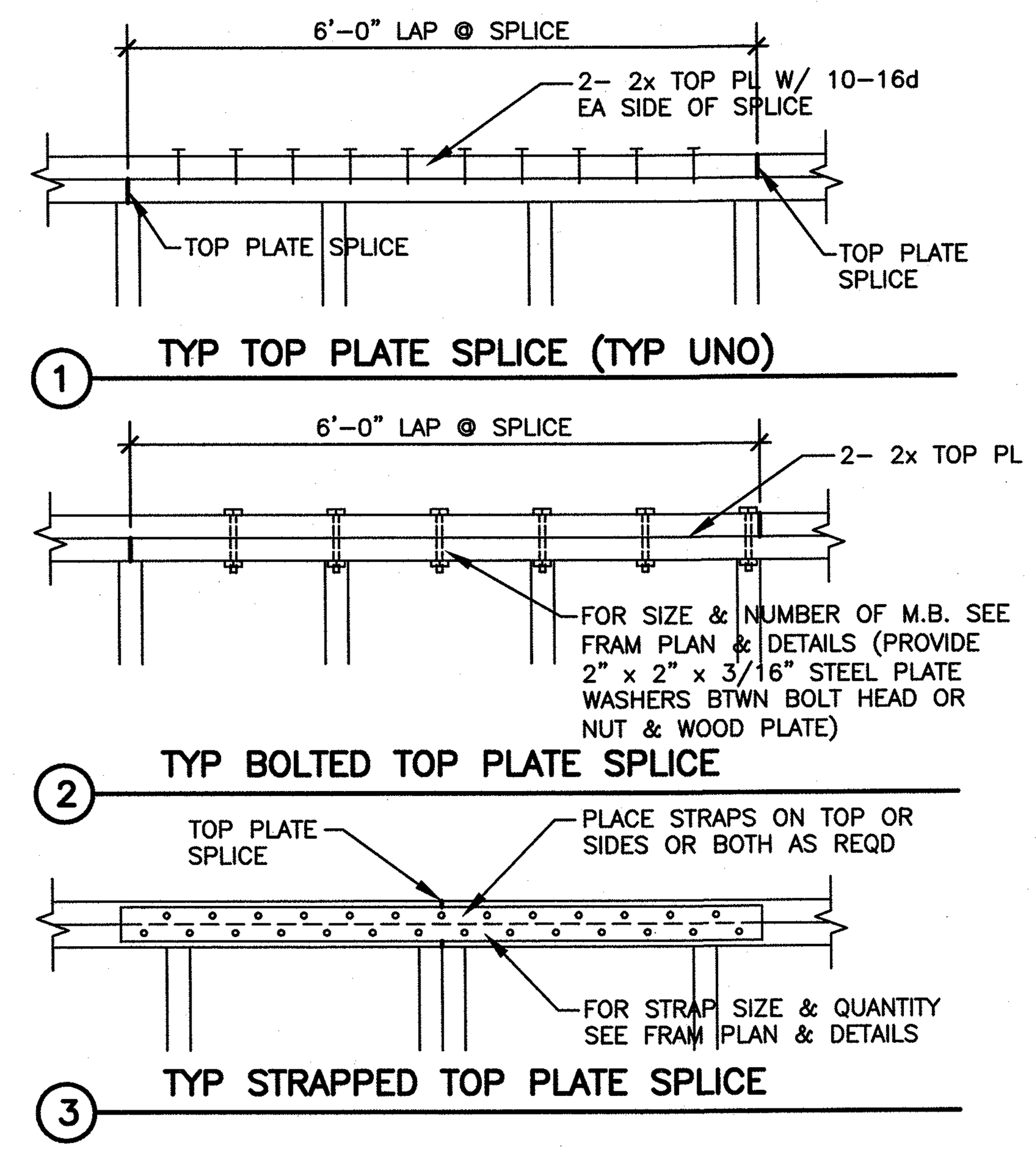


PLYWOOD PANEL SCHEDULE

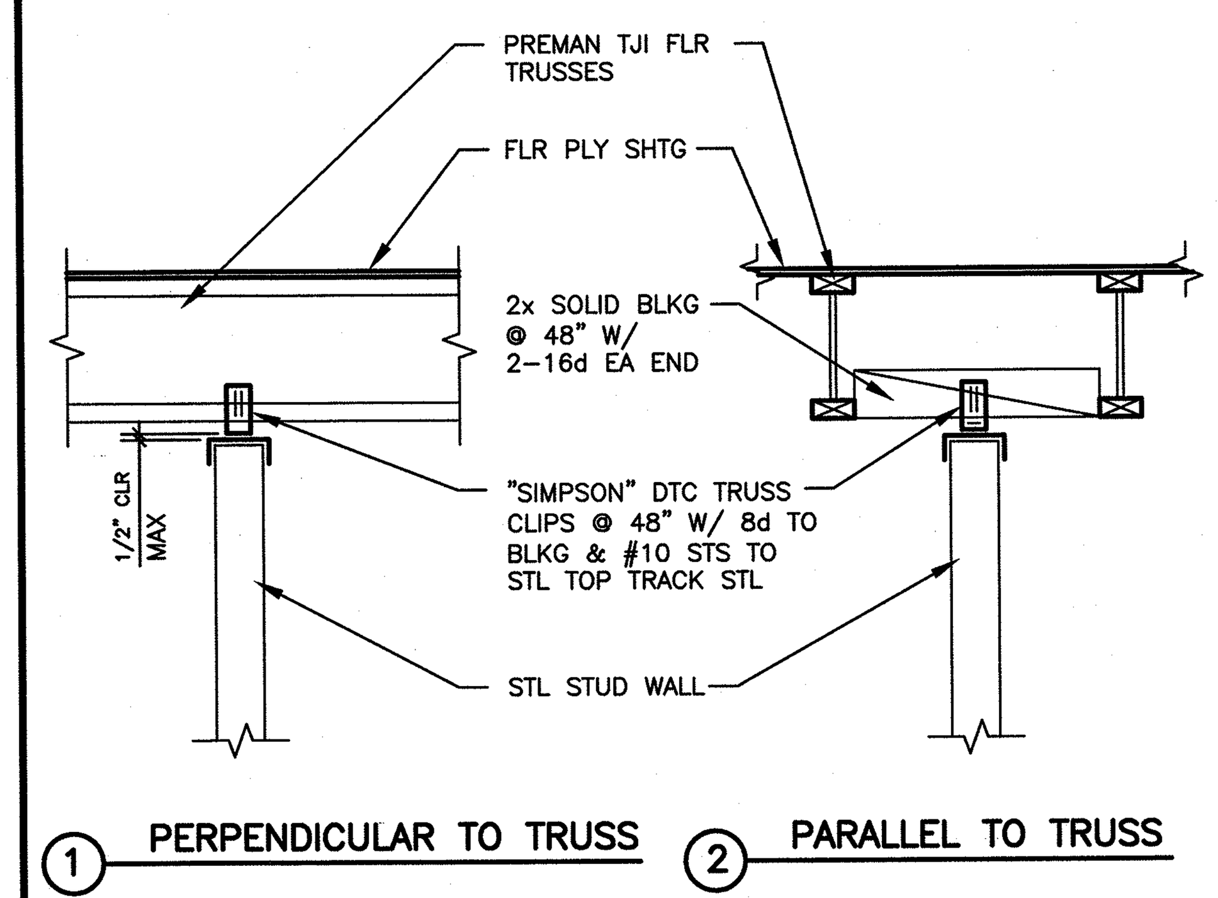
TYPE	PANEL DESIGNATION	EDGE NAILING	NAILING @ INTERM	BLKG
FLOOR	3/4" CDX	10d @ 6"	10d @ 10"	2x
FLOOR	3/4" CDX	10d @ 6"	10d @ 10"	—
ROOF	15/32" CDX	10d @ 6"	10d @ 12"	—

1 ALL NAILS SHALL BE COMMON WIRE NAILS
2 ALL PLYWOOD MUST BE MARKED "APA W/ EXT GLUE"
3 PROVIDE EDGE NAILING ALONG ALL STRUT & CHORD LINES
4 PREDRILL HOLES WHERE PLYWOOD TENDS TO SPLIT
5 MIN INDIVIDUAL PIECE: 8 SQ FT – LEAST DIMENSION = 2'-0"
6 ALL FLOOR PLYWOOD IS TO BE NAILED AND GLUED TO ALL FLOOR FRAMING

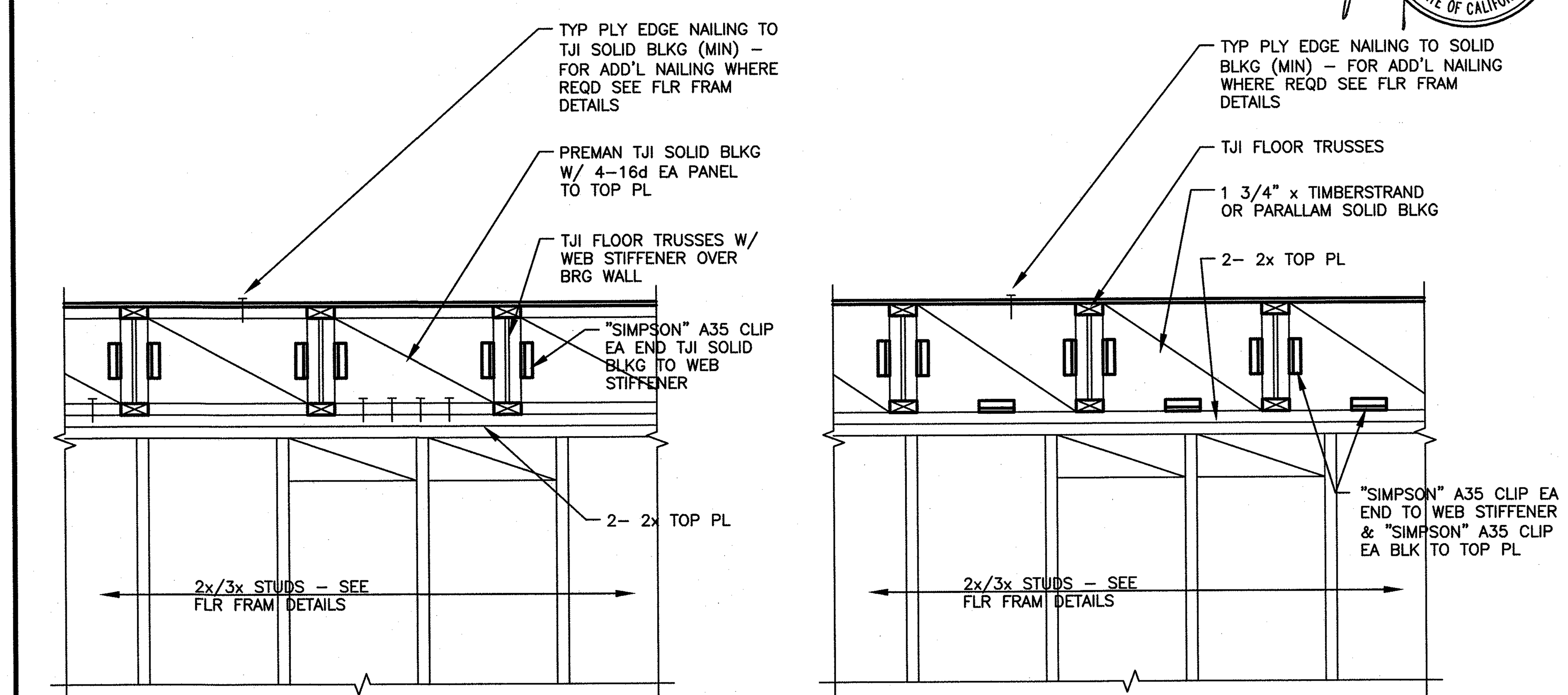
A N/A PLYWOOD SHEATHING



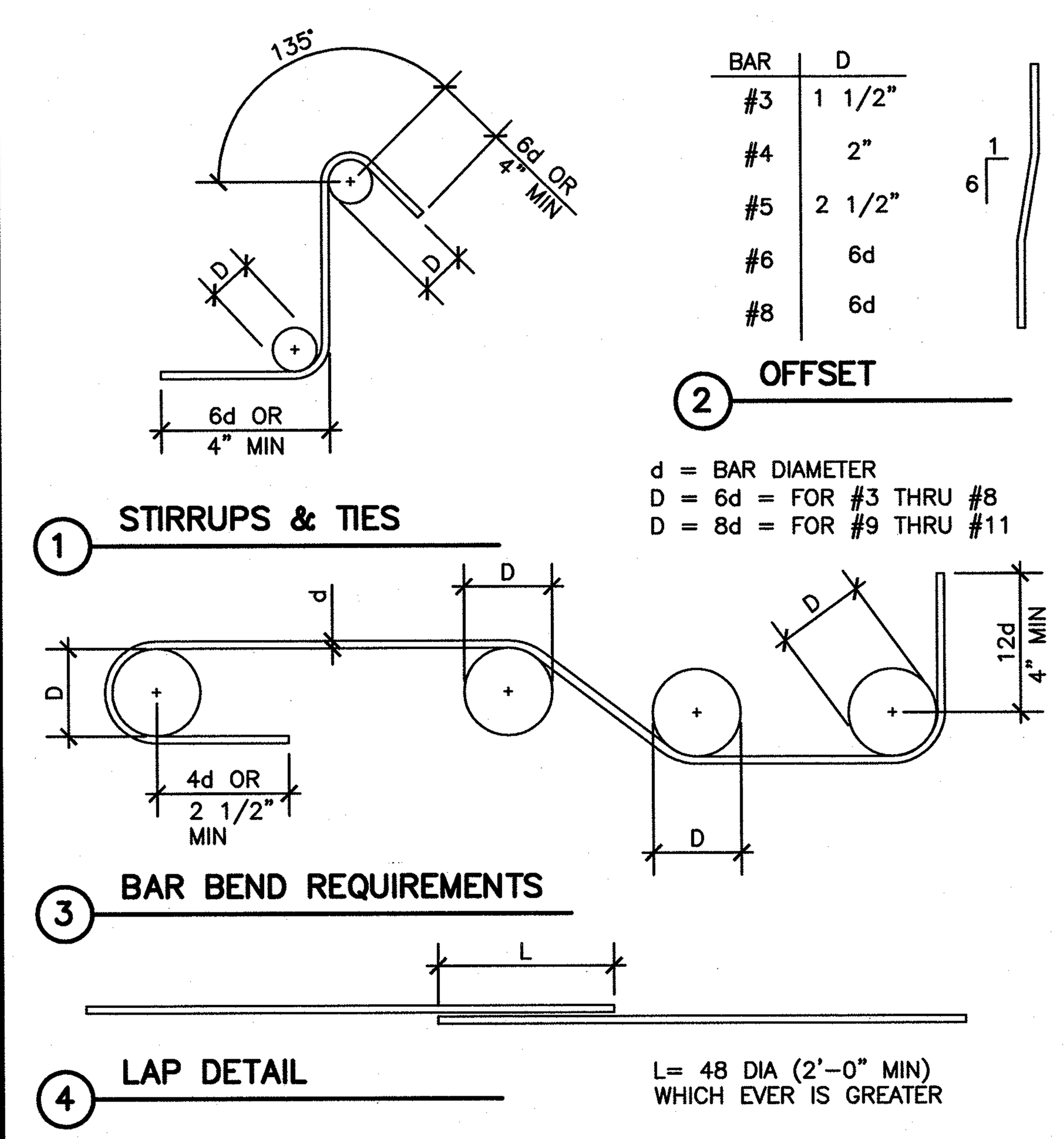
B N/A TYPICAL TOP PLATE SPLICE



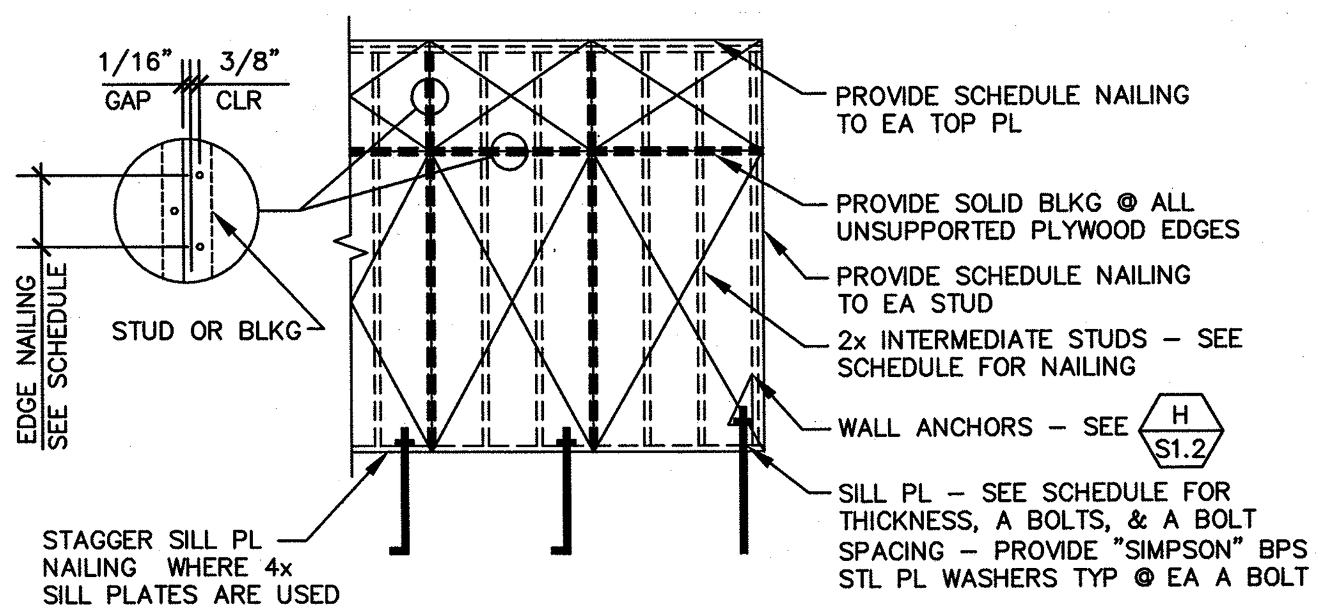
C N/A TYPICAL NON-BRG WALLS



D N/A SECTION AT TJI BLOCKING



F N/A REINFORCING DETAILS



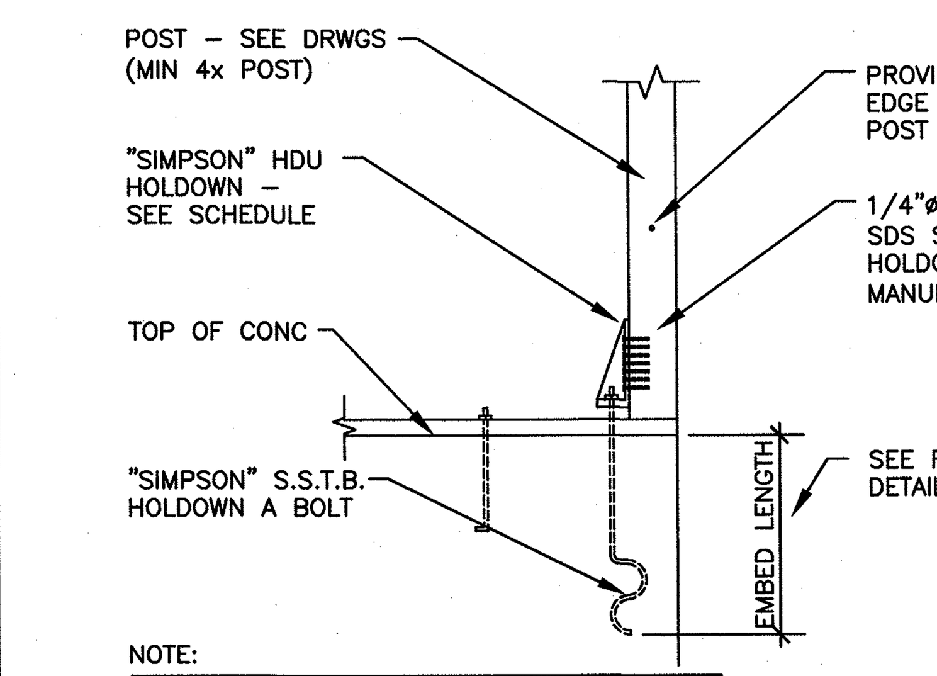
NOTES:

1 3/8" CDX PLYWOOD MAY BE SUBSTITUTED WITH 3/8" OSB STRUCT RATED APA 24/0.
2 PROVIDE SHEAR WALL NAILING AT EVERY PLYWOOD PANEL EDGE.
3 ALL SHEATHING NAILS SHALL BE COMMON WIRE TYPE.
4 SET NAILS WITHOUT CRUSHING FACE PLIES.
5 PROVIDE BLOCKING AT ALL PANEL EDGES WITH FULL STUD WIDTH BLOCKING. REFER TO SCHEDULE FOR MINIMUM STUD, BLOCKING & SILL PLATE WIDTH OR THICKNESS.
6 ALL PLYWOOD MUST BE MARKED "APA W/ EXT GLUE".
7 ALL SHEARWALL ANCHOR BOLTS ARE TO HAVE AN 8" MINIMUM EMBEDMENT. USE LONGER BOLTS WITH ADDITIONAL LENGTH PROJECTED ABOVE SLAB/FOOTING FOR 3x AND 4x SILL PLATES.
8 MIN INDIVIDUAL PIECE: 8 SQ FT – LEAST DIMENSION = 1'-4"
9 WHERE PLYWOOD IS APPLIED TO EACH FACE OF WALL STAGGER PLYWOOD SPLICES TO DIFFERENT STUDS.
10 PRESSURE TREATED SILL PLATES ARE CORROSIVE THEREFORE ALL NAILS USED TO ATTACH THE PLYWOOD TO THE PRESSURE TREATED SILL PLATES ARE TO BE HOT DIPPED GALVANIZED.
11 INDICATES CLIP SPACING REQUIREMENT. FOR CLIP DETAILING / LOCATION REFER TO STRUCTURAL DETAIL SHEET.

PLYWOOD PANEL SCHEDULE

TYPE	SHEATHING	SHEARWALL NAILING (EDGE)	SHEARWALL NAILING (FIELD)	BLKG & STUDS	SILL PL	SILL PL ANCHORAGE		CLIP SPACING (11)		CAPACITY
						ANCHOR BOLTS	NAILING/SDS	LS50/RBC	LTP4/A35	
A	3/8" CDX PLYWOOD	8d @ 6"	8d @ 12"	2x	2x	5/8"ø @ 48"	16d @ 9"	20"	30"	260 plf
B	3/8" CDX PLYWOOD	8d @ 4"	8d @ 12"	2x	2x	5/8"ø @ 48"	16d @ 6"	14"	22"	350 plf
C	3/8" CDX PLYWOOD	8d @ 3"	8d @ 12"	3x	3x	5/8"ø @ 32"	6" SDS @ 6"	10"	16"	490 plf
D	3/8" CDX PLYWOOD	8d @ 2"	8d @ 8"	3x	3x	5/8"ø @ 32"	6" SDS @ 5"	8"	12"	640 plf
E	15/32" CDX PLYWOOD	10d @ 2"	10d @ 8"	3x	3x	5/8"ø @ 16"	6" SDS @ 4"	7"	10"	770 plf
F	1/2" STR I RATED PLY	10d @ 2"	10d @ 12"	3x	3x	5/8"ø @ 16"	6" SDS @ 3"	6"	9"	870 plf
G	3/8" CDX PLY (2 SIDES)	8d @ 3"	8d @ 12"	3x	3x	5/8"ø @ 16"	6" SDS @ 3"	5"	8"	980 plf
H	3/8" CDX PLY (2 SIDES)	8d @ 2"	8d @ 8"	4x	4x	5/8"ø @ 16"	8" SDS @ 2"	4"	6"	1280 plf
J	15/32" CDX PLY (2 SIDES)	10d @ 2"	10d @ 8"	4x	4x	5/8"ø @ 12"	8" SDS @ 2"	3"	5"	1540 plf

G N/A PLYWOOD SHEARWALL



NOTE:

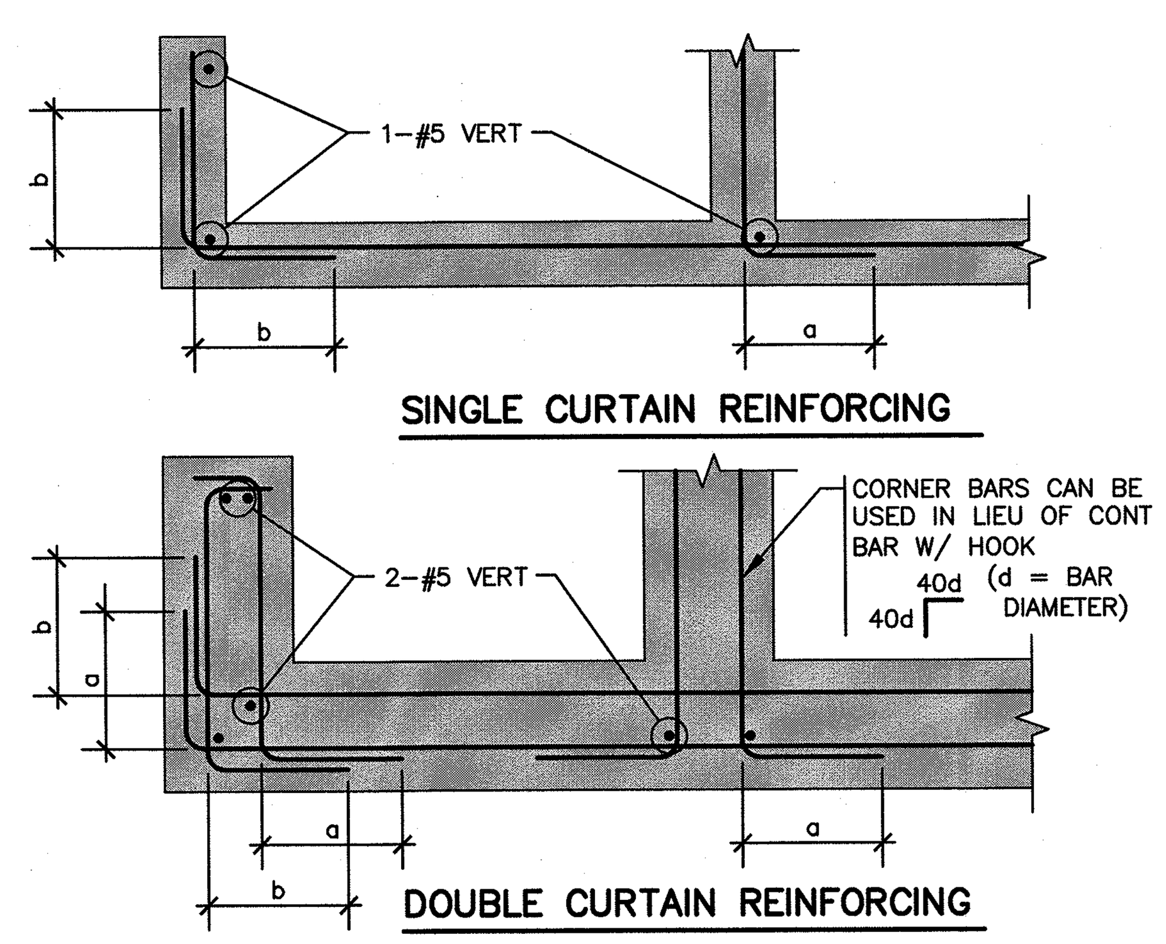
1 INSTALL HOLDOWN PER MANUFACTURER'S RECOMMENDATIONS

2 ANCHOR BOLT NUT IS TO BE FINGER TIGHT WITH 1/3 TO 1/2 TURN WITH A WRENCH

HDU HOLDOWN SCHEDULE

SYMBOL	TYPE	A BOLT	EMBED LENGTH	SDS SCREWS (BY MANUF)
1	HDU2	5/8" DIA	13"	6 – 1/4" DIA x 2 1/2"
2	HDU4	5/8" DIA	17"	10 – 1/4" DIA x 2 1/2"
3	HDU5	5/8" DIA	21"	14 – 1/4" DIA x 2 1/2"
4	HDU8	7/8" DIA	25"	20 – 1/4" DIA x 2 1/2"

H N/A TYPICAL HOLDOWN DETAIL



BAR SIZE		#4	#5	#6	#7	#8	#9
GRADE 40	a	1'-0"	1'-0"	1'-4"	1'-10"	2'-5"	3'-0"
	b	1'-9"	1'-9"	2'-4"	3'-2"	4'-2"	5'-2"
GRADE 60	a	1'-0"	1'-5"	2'-0"	2'-9"	3'-7"	4'-6"
	b	1'-9"	2'-6"	3'-6"	4'-9"	6'-2"	7'-8"

J N/A PLAN AT TYP REINFORCING

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By
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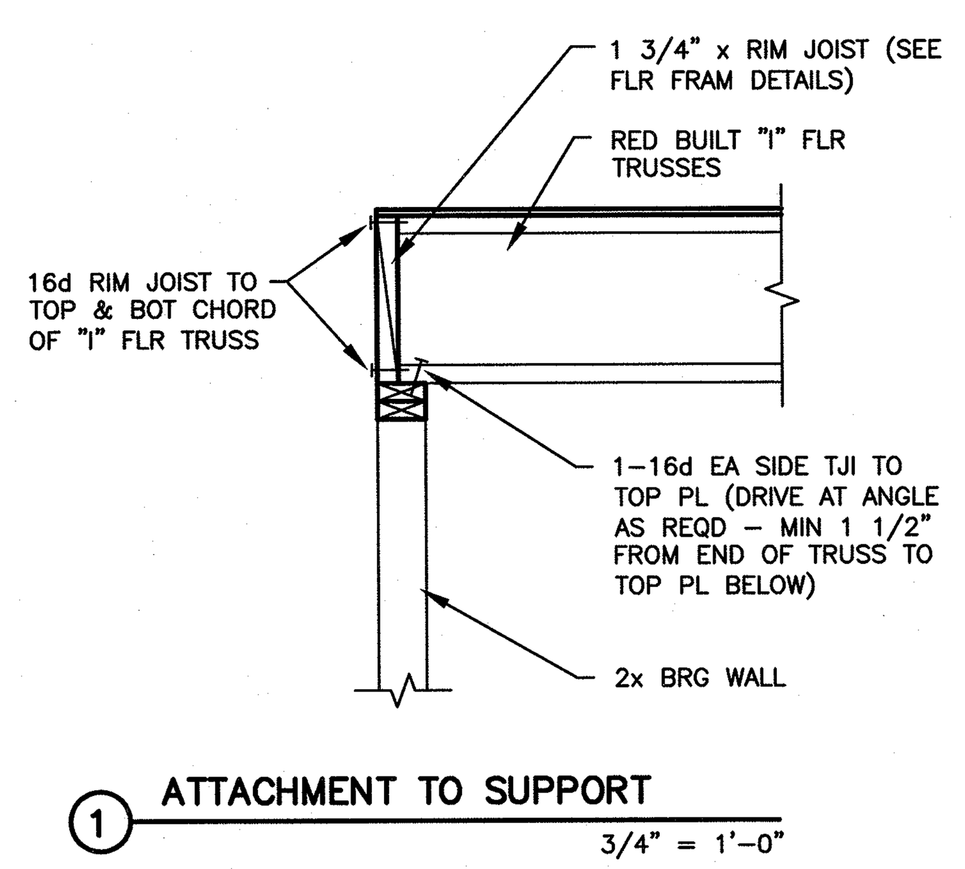
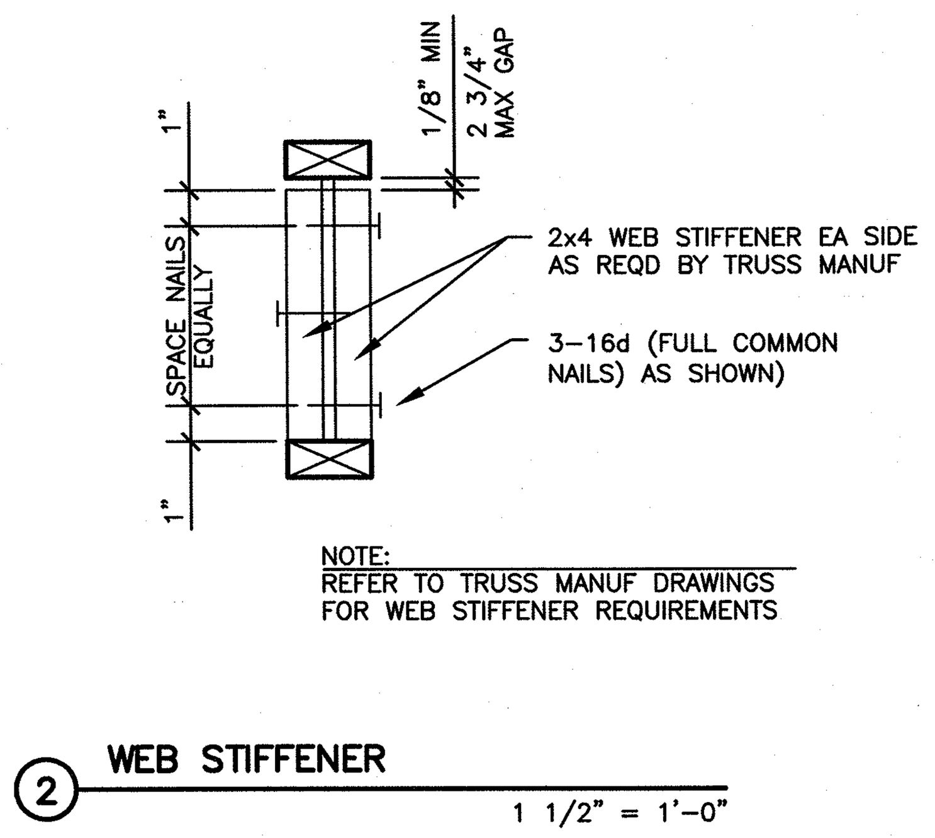
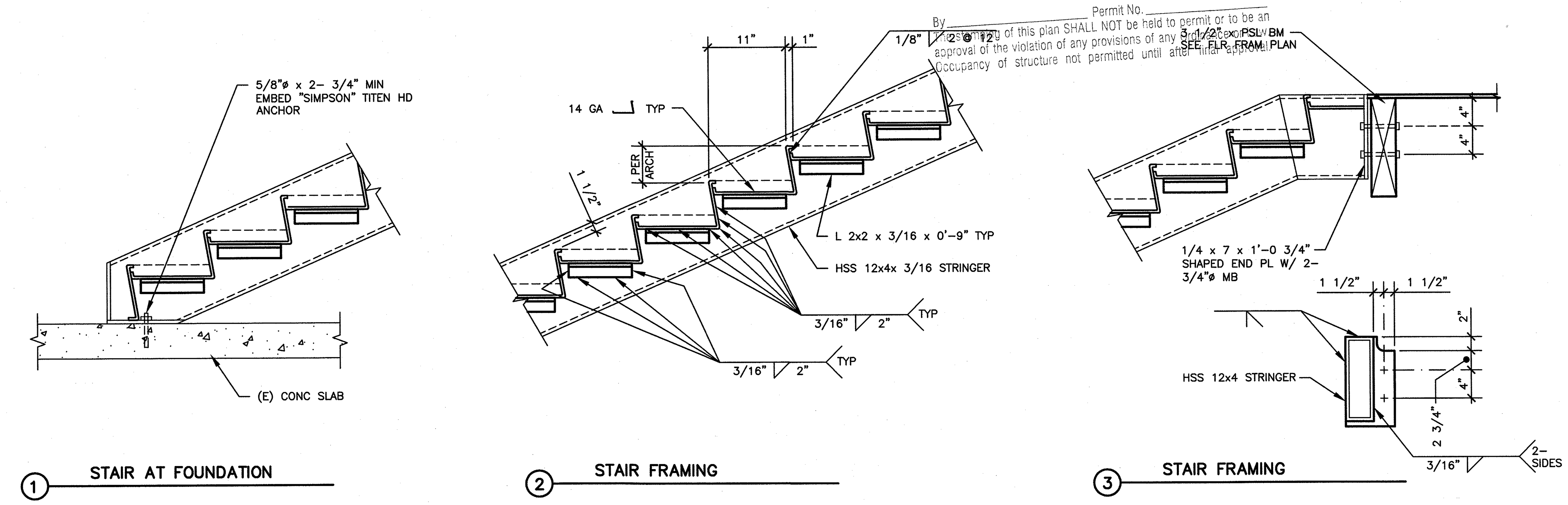




K N/A SECTION AT "I" TRUSS BEARING REQUIRMENTS



L N/A TYPICAL STAIR DETAILS

L Street Architects
Limited
1414 L Street
Modesto, California 95354
209-575-1415 209-575-4374 fax
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12-18-14
REGISTERED PROFESSIONAL ENGINEER STRUCTURAL STATE OF CALIFORNIA
Exp. 12-31-14

PELTON WYLIE ENGINEERING, INC
2813 COFFEE ROAD SUITE D1
MODESTO, CALIFORNIA 95355
TEL: (209)575-9619 FAX: (209)575-0274

PROPOSED TENANT IMPROVEMENT FOR:
CENTER FOR HUMAN SERVICES
2000 W BRIGGSMORE, SUITE I
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REVISIONS:

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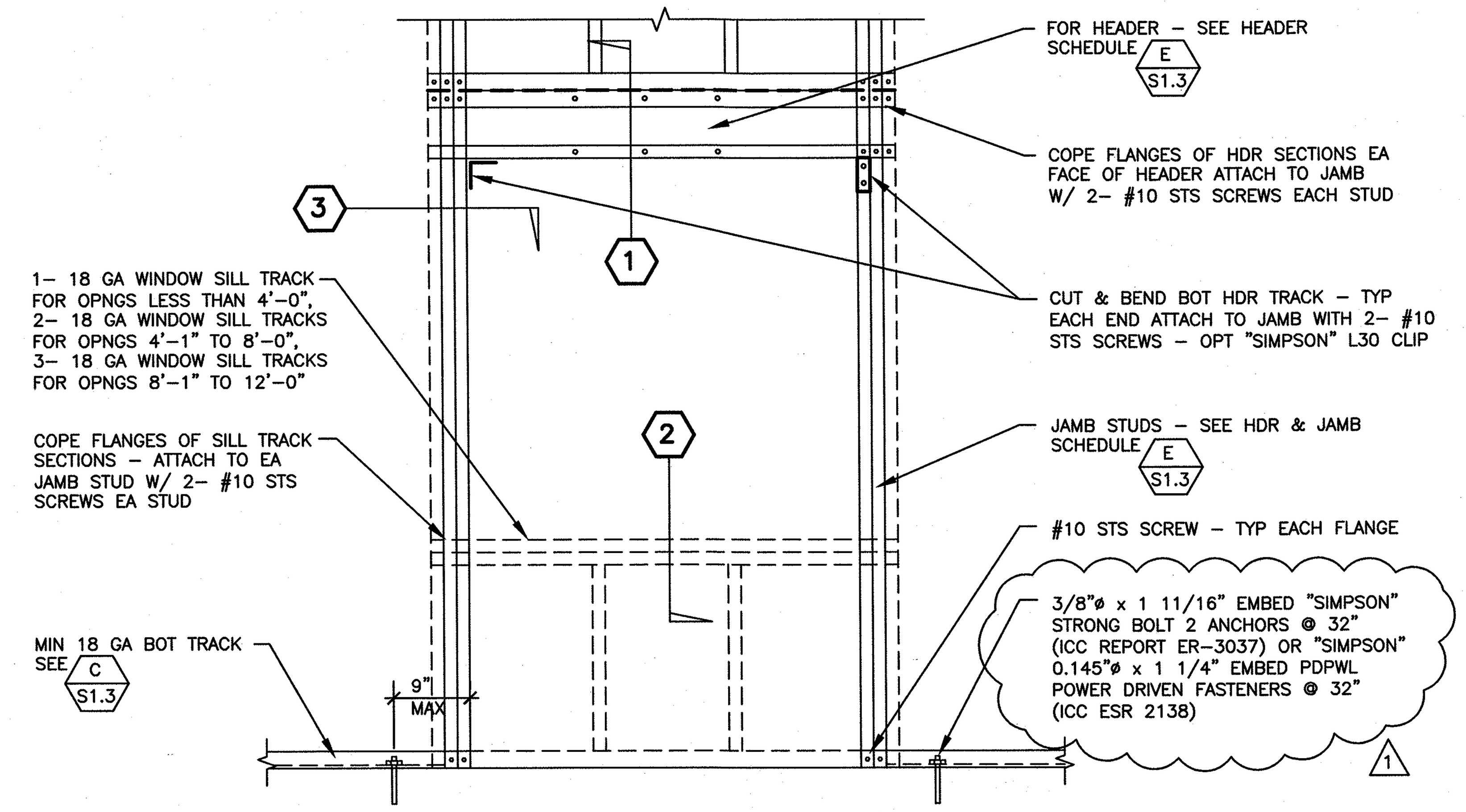
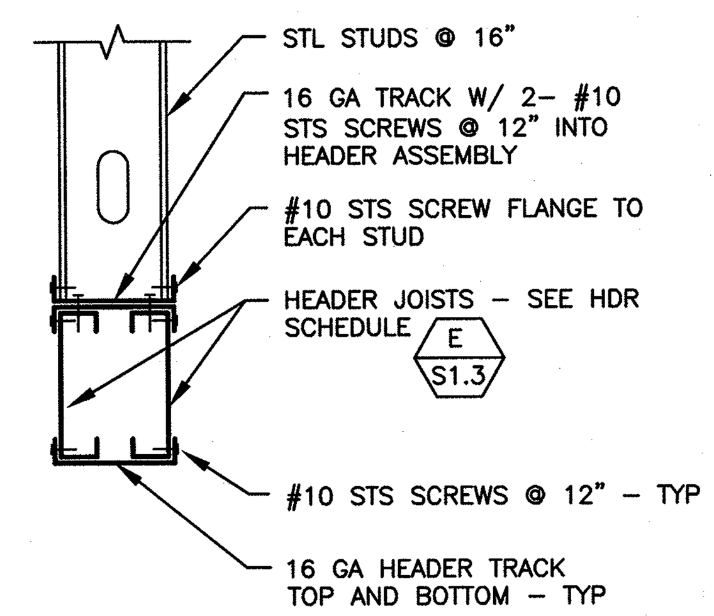
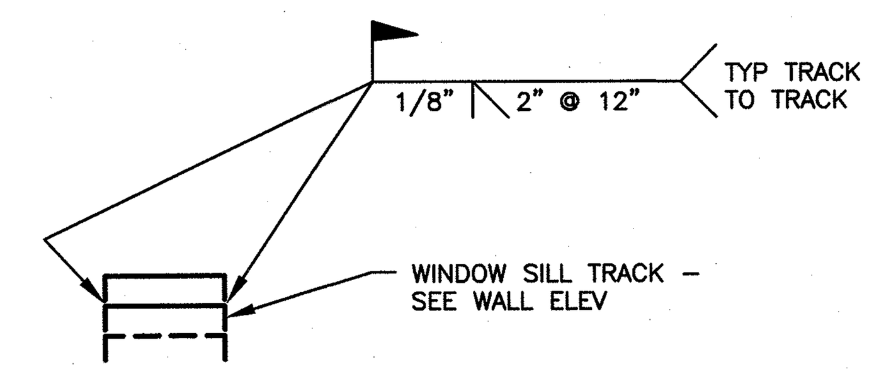
TYPICAL DETAILS

S1.2

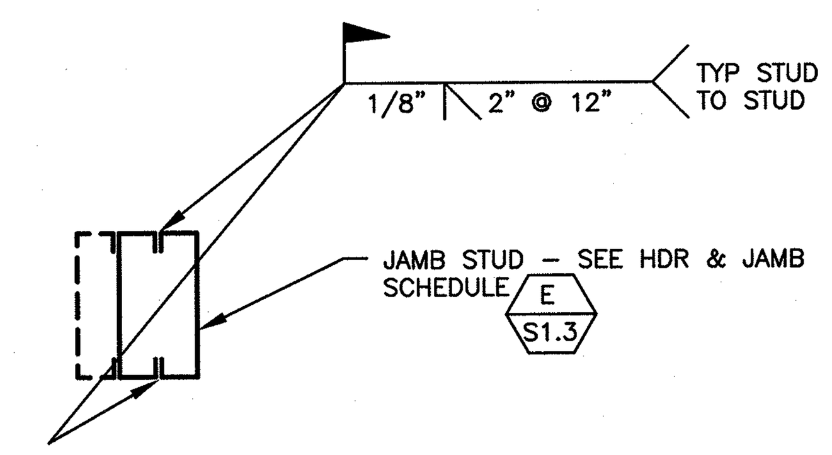




1 TYPICAL BOXED HEADER AT 3'–1" TO 12'–0" OPENINGS

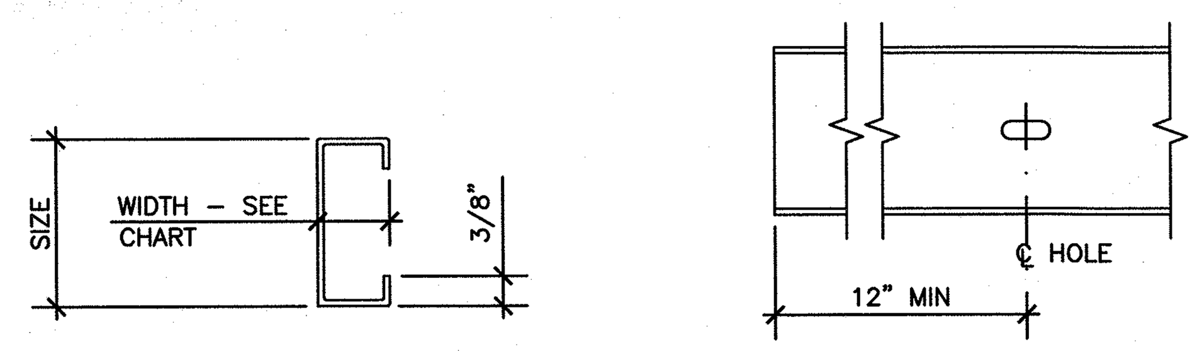


2 SECTION



3 SECTION

A NA TYPICAL NON-STRUCTURAL STEEL STUD WALL AT FRAMED OPENING

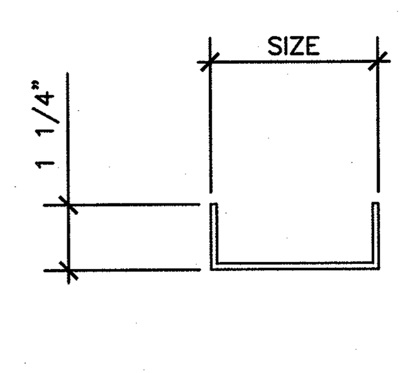


NOTE: SECTION PROPERTIES INDICATED ARE BASED ON STUDS MANUFACTURED BY DIETRICH INDUSTRIES

PHYSICAL STRUCTURAL PROPERTIES - C STUDS / JOISTS

SIZE (IN)	WIDTH	GAGE	Fy (KSI)	WT (#/FT)	AREA (IN^2)	Ix (IN^4)	Sx (IN^3)	Rx (IN)	Iy (IN^4)	Sy (IN^3)	Ry (IN)
C 3 1/2"	1 3/8"	18	33	0.982	0.300	0.568	0.325	1.376	0.075	0.078	0.499
C 3 1/2"	1 3/8"	20	33	0.764	0.233	0.446	0.245	1.383	0.060	0.062	0.505
C 6"	1 3/8"	20	33	1.047	0.320	1.597	0.513	2.234	0.070	0.065	0.466
C 6"	1 3/8"	18	33	1.351	0.413	2.043	0.681	2.224	0.087	0.082	0.460
C 8"	1 3/8"	18	33	1.646	0.503	4.135	1.034	2.867	0.893	0.083	0.430
C 6"	1 3/8"	16	50	1.821	0.557	2.862	0.928	2.268	0.181	0.149	0.570

B NA STUD SECTION PROPERITES



NOTE: SECTION PROPERTIES INDICATED ARE BASED ON STUDS MANUFACTURED BY DIETRICH INDUSTRIES

PHYSICAL STRUCTURAL PROPERTIES - TRACK

SIZE (IN)	GAGE	Fy (KSI)	WT (#/FT)	AREA (IN^2)	Ix (IN^4)	Sx (IN^3)	Rx (IN)	Iy (IN^4)	Sy (IN^3)	Ry (IN)
3 1/2"	16	33	1.094	0.334	0.611	0.307	1.375	0.047	0.049	0.377
3 1/2"	18	33	0.873	0.267	0.462	0.226	1.374	0.038	0.039	0.379
6"	16	33	1.557	0.476	2.206	0.671	2.183	0.054	0.052	0.336
6"	18	33	1.242	0.379	1.688	0.505	2.183	0.043	0.041	0.338

C NA TRACK SECTION PROPERTIES

INTERIOR NON-STRUCTURAL STEEL STUD SCHEDULE

MEMBER SIZE	GAGE	MAXIMUM HEIGHT (FEET)	SPACING
C 3 1/2" x 1 3/8"	18	14'–0"	24"
C 3 1/2" x 1 3/8"	20	13'–0"	24"
C 3 1/2" x 1 3/8"	18	16'–0"	16"
C 3 1/2" x 1 3/8"	20	15'–0"	16"
C 6" x 1 3/8"	16	22'–0"	24"
C 6" x 1 3/8"	18	21'–0"	24"
C 6" x 1 3/8"	20	19'–0"	24"
C 6" x 1 3/8"	16	25'–0"	16"
C 6" x 1 3/8"	18	23'–0"	16"
C 6" x 1 3/8"	20	21'–0"	16"

CEILING JOIST SCHEDULE

MEMBER SIZE	GAGE	MAXIMUM SPAN	SPACING
3 1/2" x 1 3/8"	18	10'–0"	16"
3 1/2" x 1 3/8"	20	8'–0"	16"
6" x 1 3/8"	16	16'–0"	16"
6" x 1 3/8"	18	14'–0"	16"
6" x 1 3/8"	20	12'–0"	16"

D NA CLG JOIST & WALL STUD SCHEDULE

HEADER AND JAMB STUD SCHEDULE

SPAN	QUANTITY & MIN GA OF JAMB STUDS	HEADER (1)
UP TO 4'–0"	2– C 3 1/2" x 1 3/8" x 20 GA STUDS OR 2– C 6" x 1 3/8" x 20 GA STUDS	2– C 3 1/2" x 1 3/8" x 18 GA
4'–1" TO 8'–0"	2– C 3 1/2" x 1 3/8" x 18 GA STUDS OR 2– C 6" x 1 3/8" x 18 GA STUDS	2– C 6" x 1 3/8" x 18 GA
8'–1" TO 12'–0"	3– C 3 1/2" x 1 3/8" x 18 GA STUDS OR 2– C 6" x 1 3/8" x 18 GA STUDS	2– C 8" x 1 3/8" x 18 GA

NOTE:

1 HEADER TRACK TOP AND BOTTOM IS TO BE A MINIMUM 16 GA x WIDTH OF FRAMED WALL.

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DEC 30 2014

By__________ Permit No.__________

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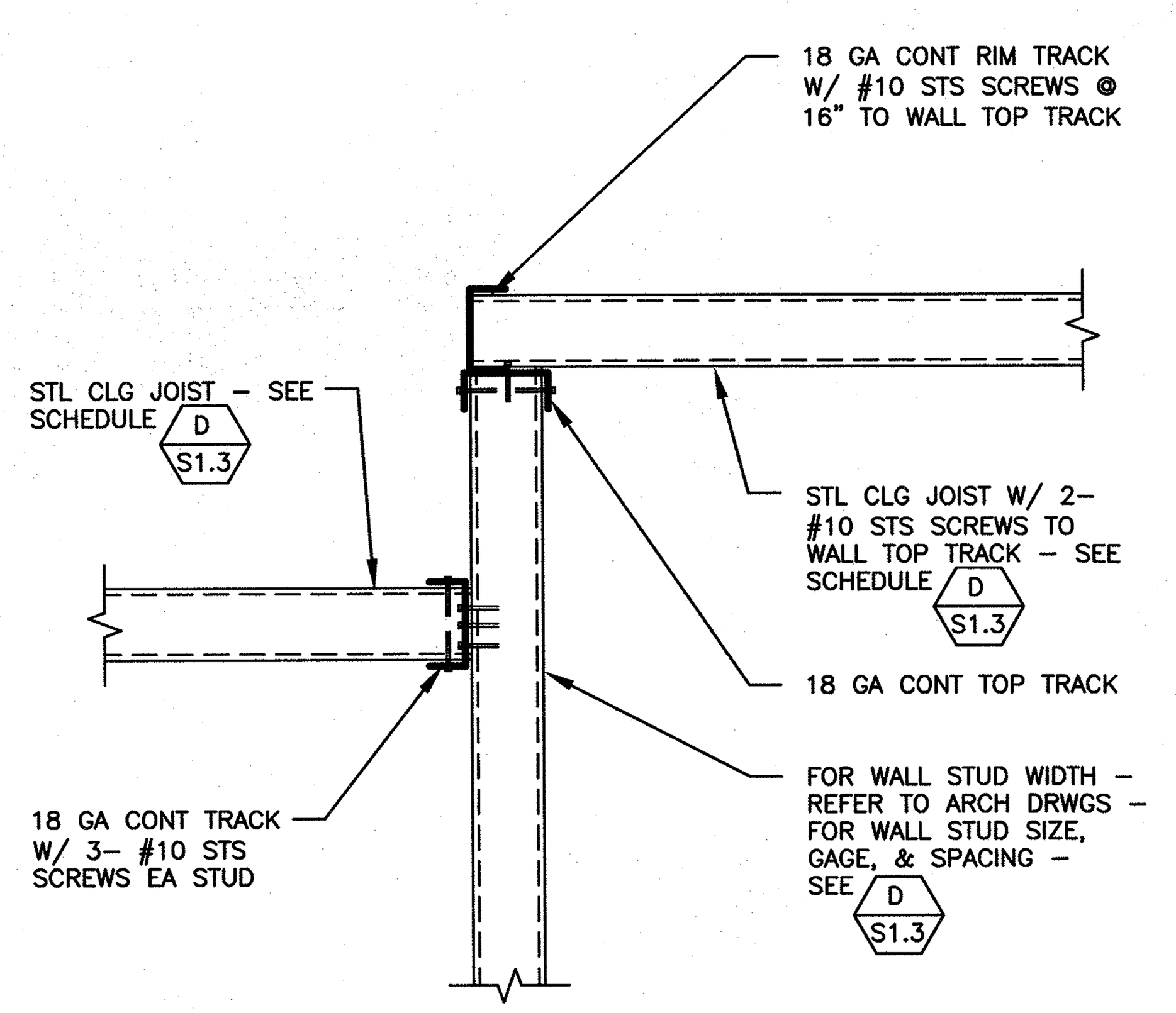
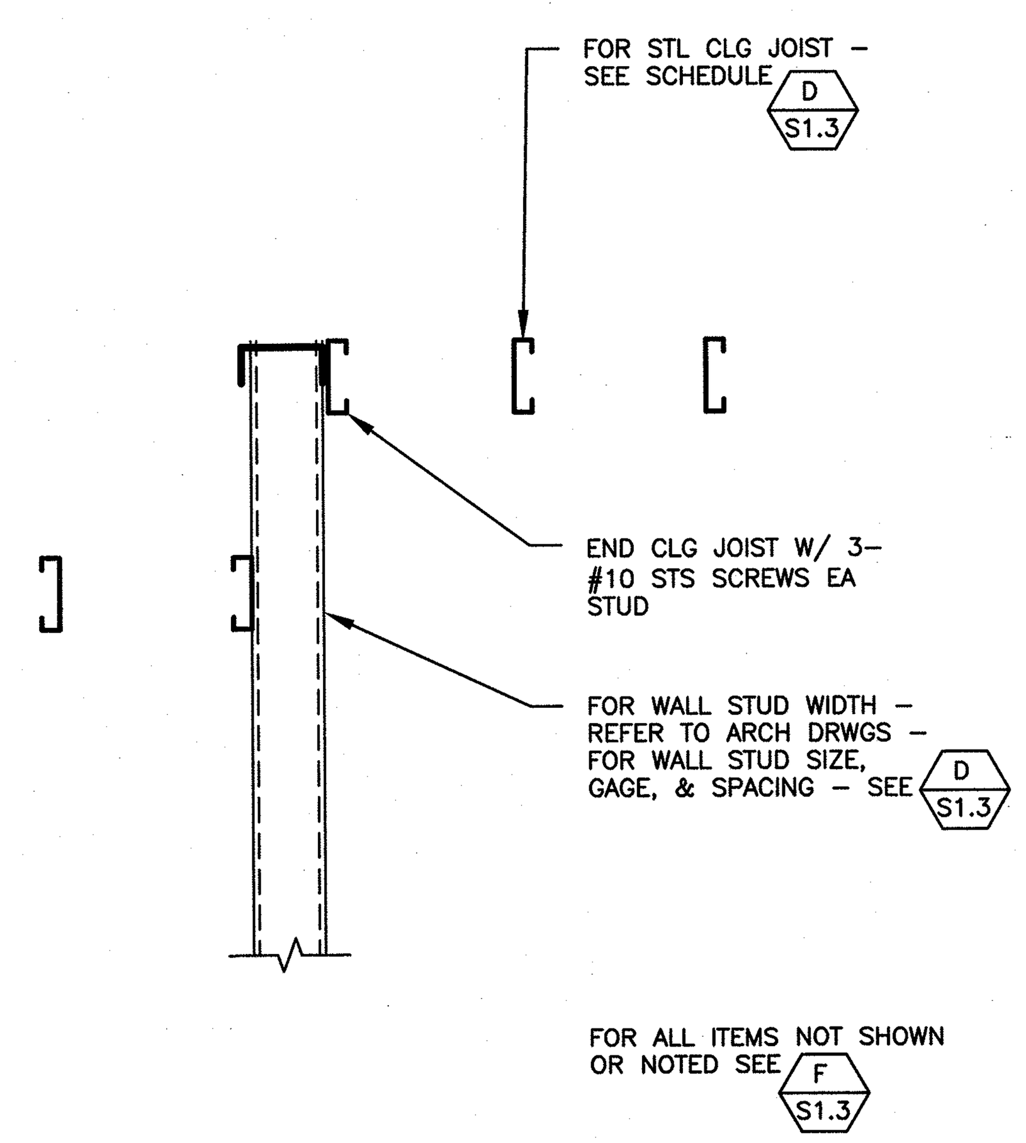
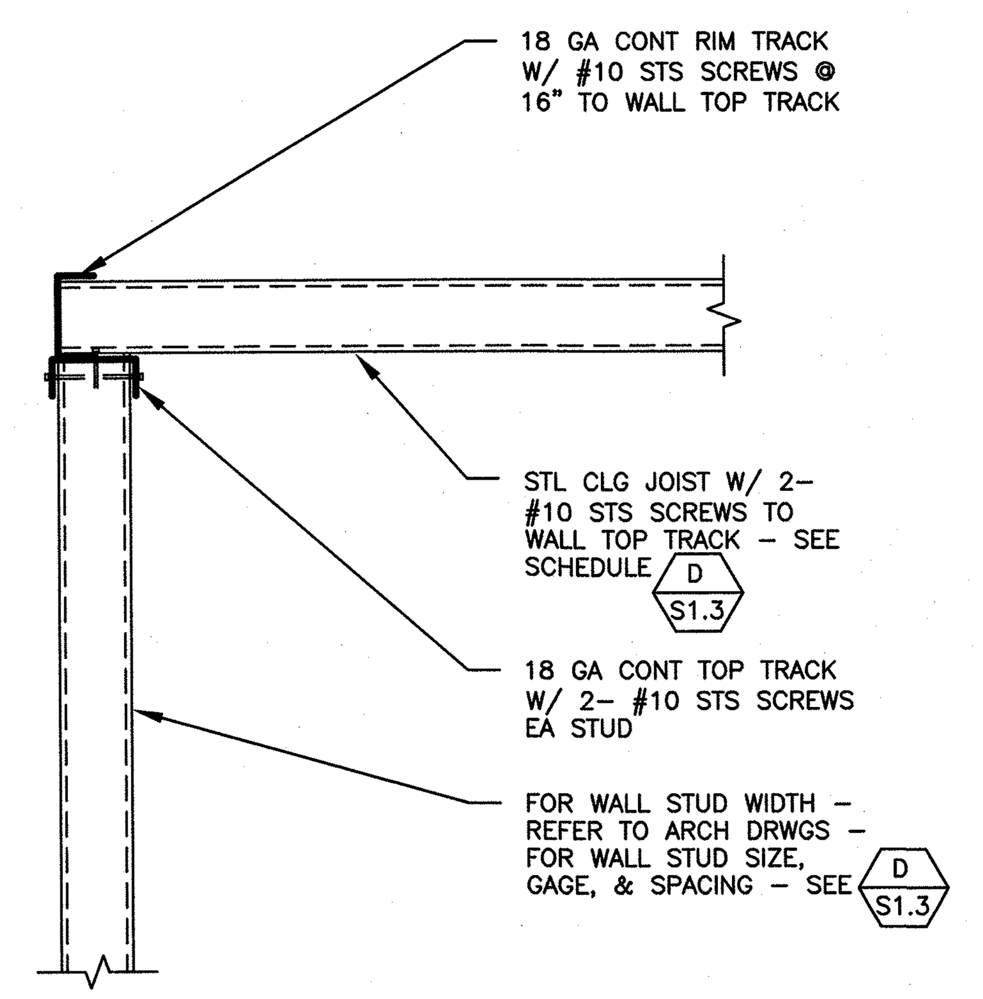
E NA STL HDR & JAMB STUD SCHEDULE



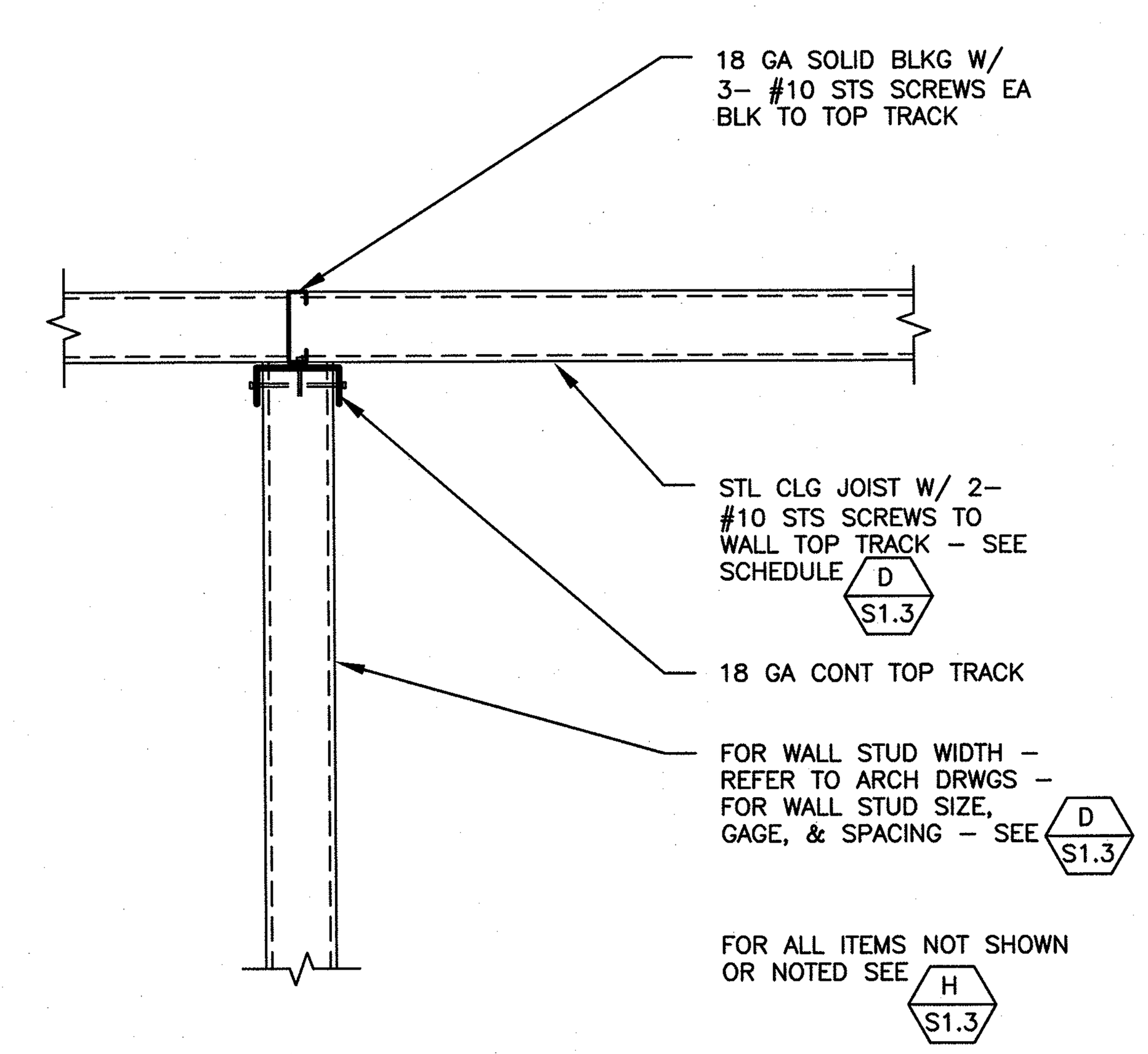
F 3/4" SECTION AT CEILING FRAMING



G 3/4" SECTION AT CEILING FRAMING



H 3/4" SECTION AT CEILING FRAMING



J 3/4" SECTION AT CEILING FRAMING

L Street Architects Limited
1414 L Street
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209-575-1415 209-575-4374 fax
www.lstreetarchitects.com

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PROPOSED TENANT IMPROVEMENT FOR:

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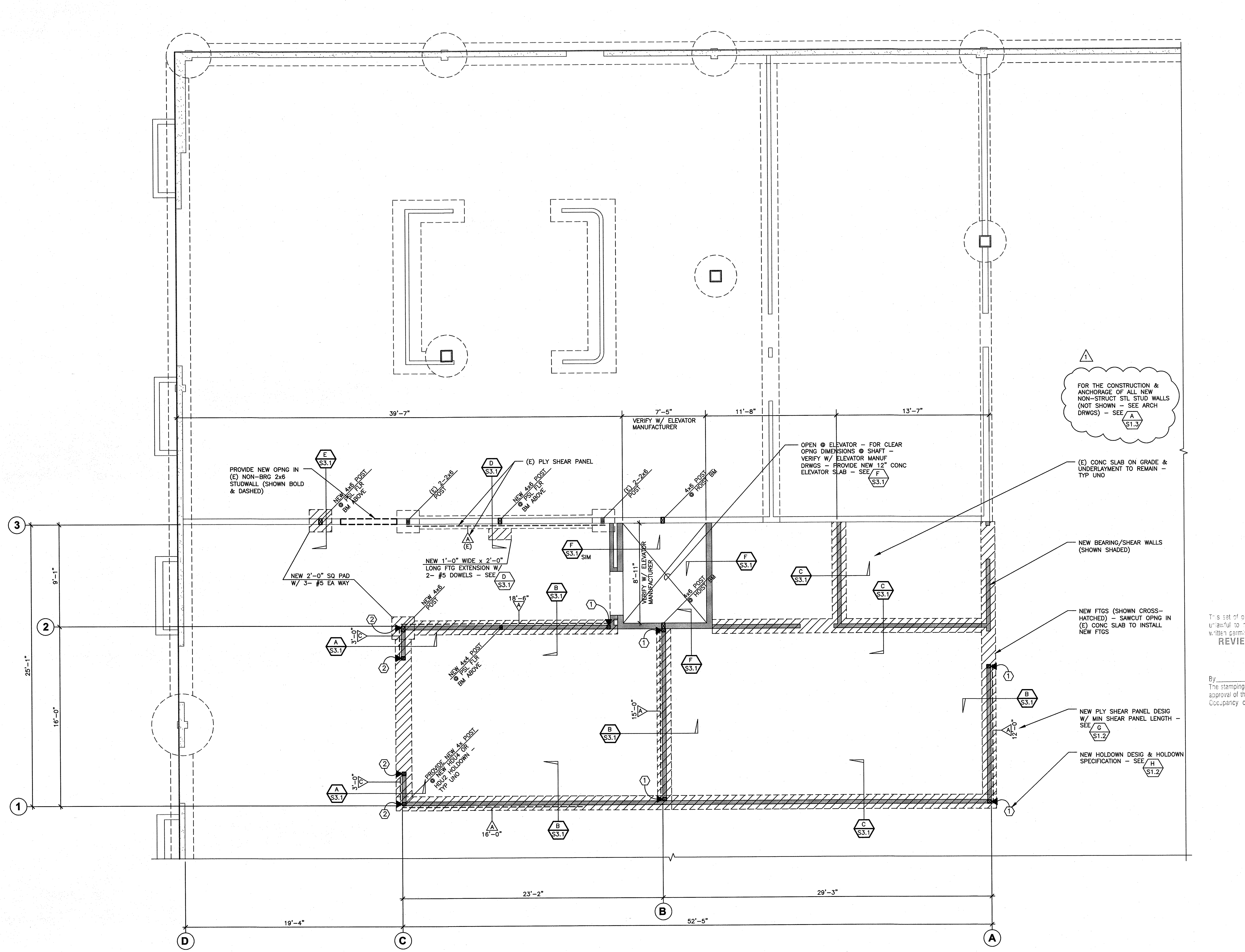
NON–STRUCT WALL AND CEILING DETAILS

S1.3



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12-12-14

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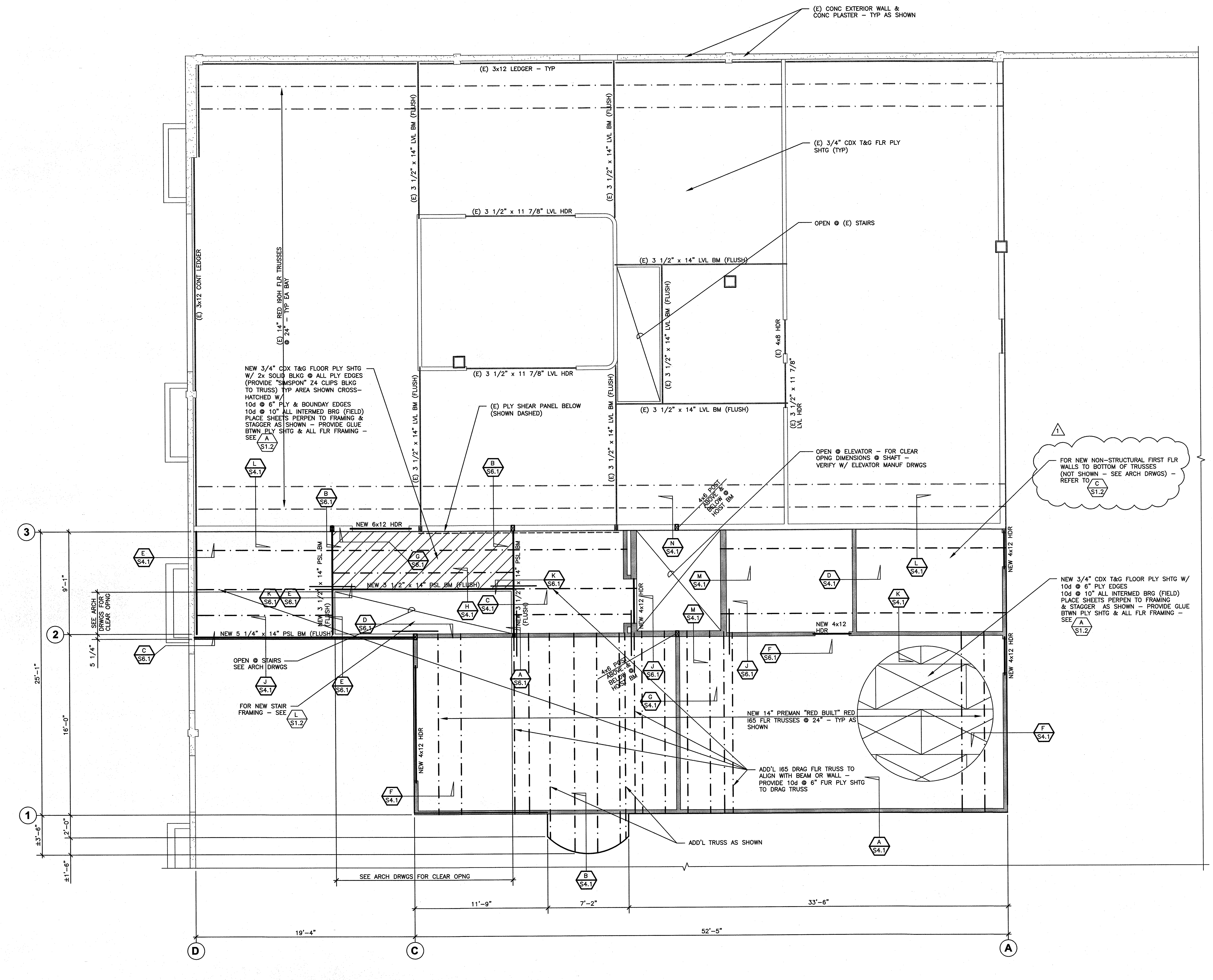
ENLARGED FOUNDATION PLAN

S2.1

A 1/4" ENLARGED FOUNDATION PLAN



A 1/4" **ENLARGED FLOOR FRAMING PLAN**

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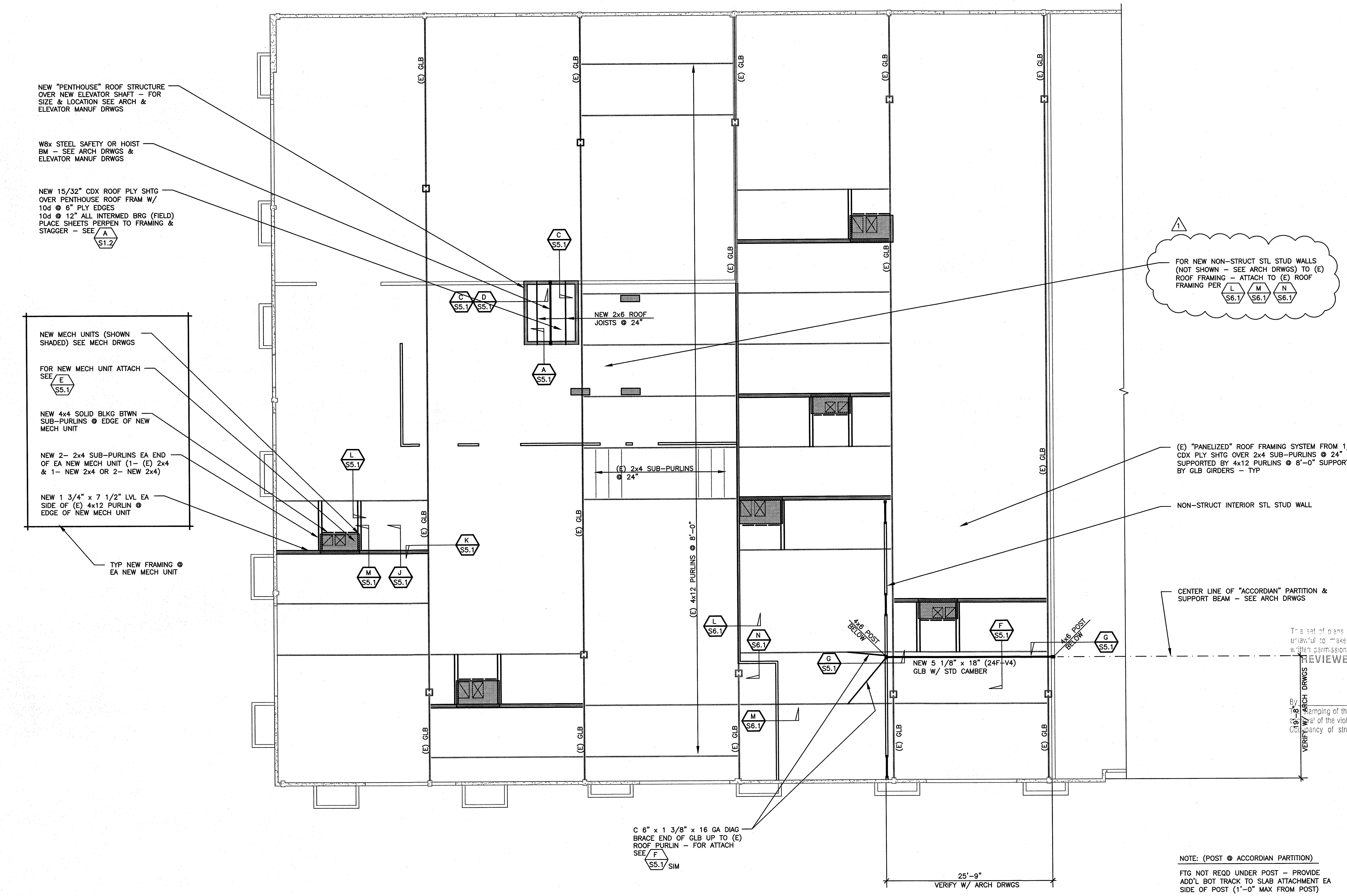
ENLARGED FLOOR FRAMING PLAN

S2.2



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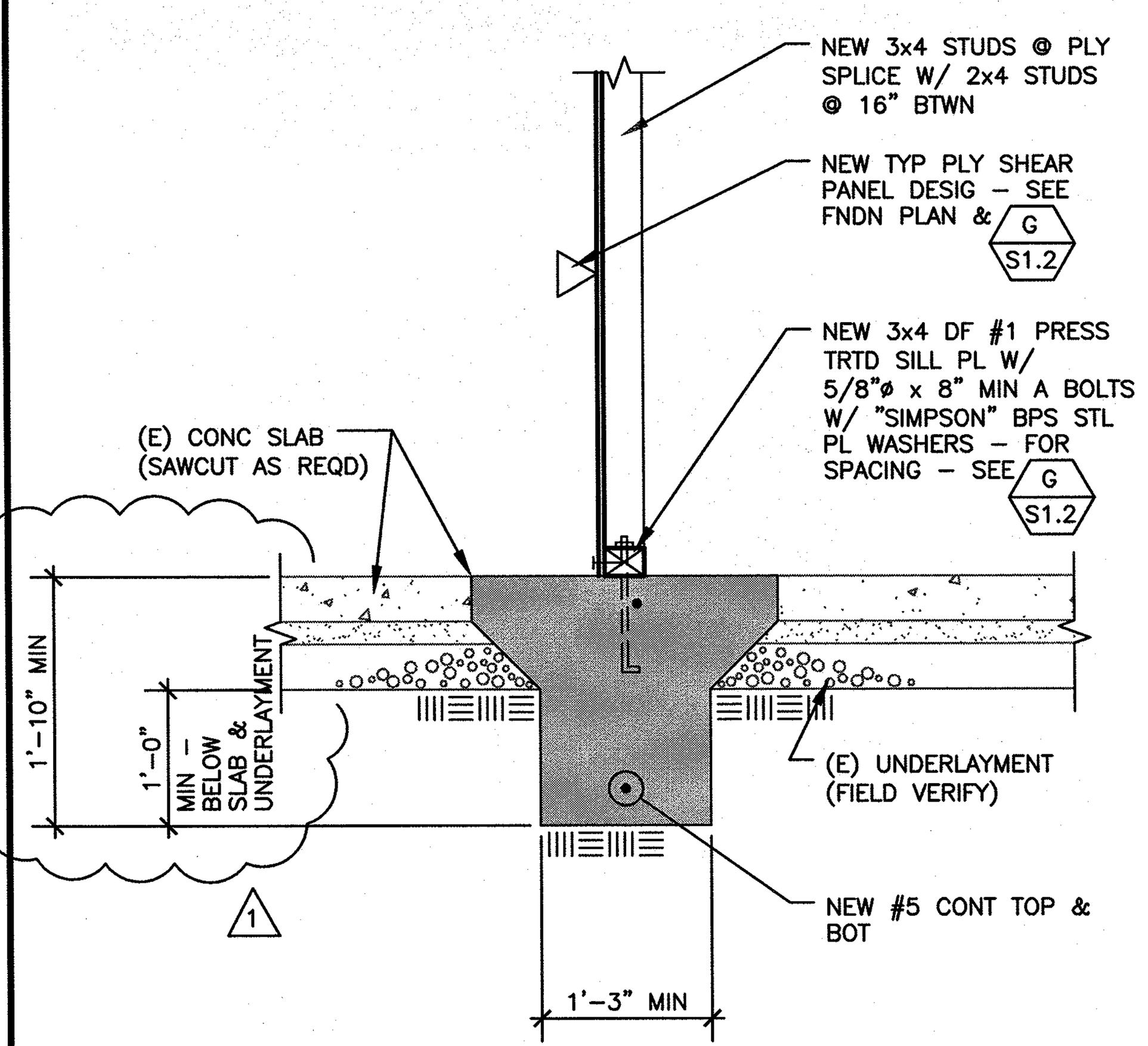
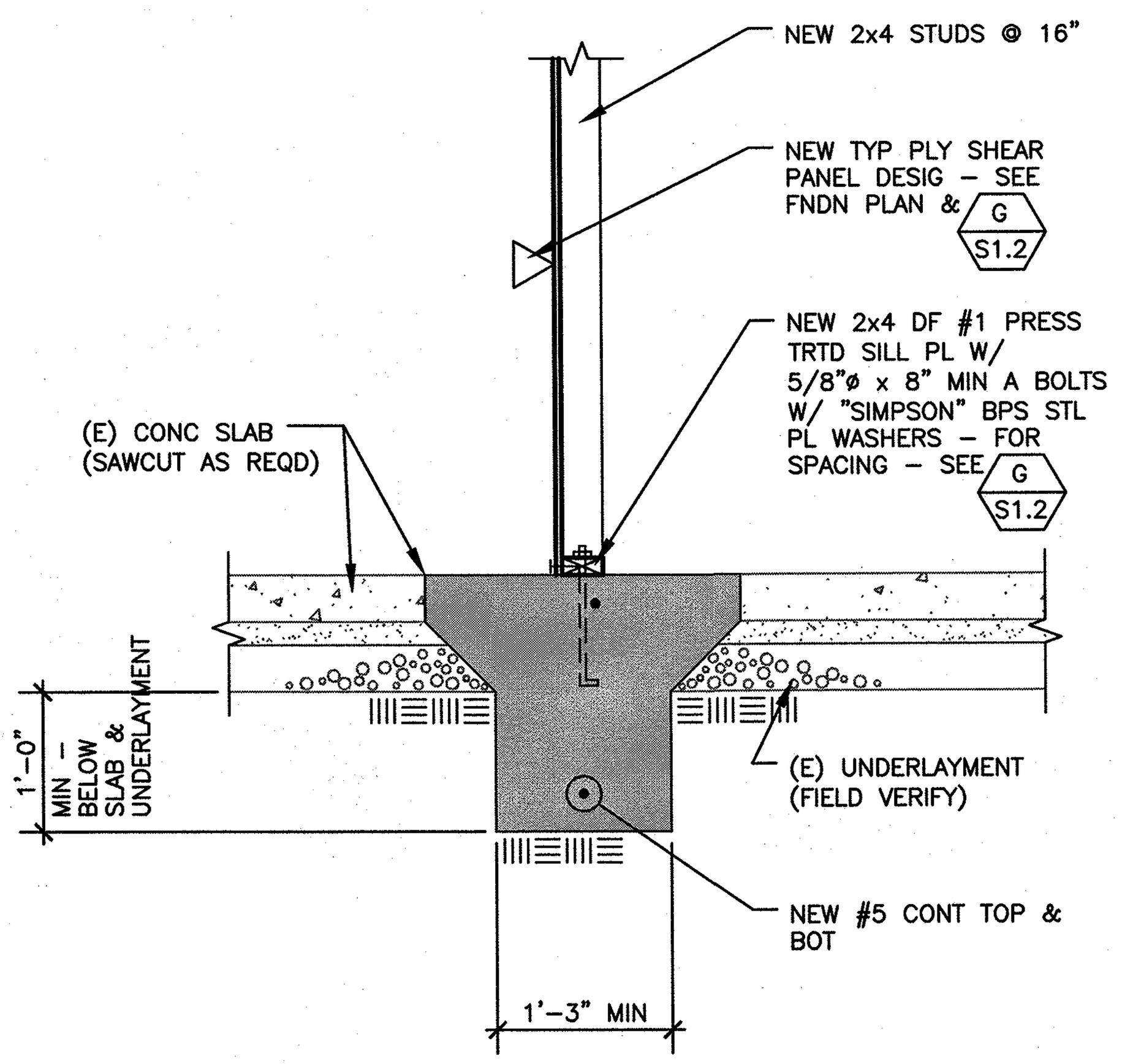
PARTIAL ROOF FRAMING PLAN

S2.3

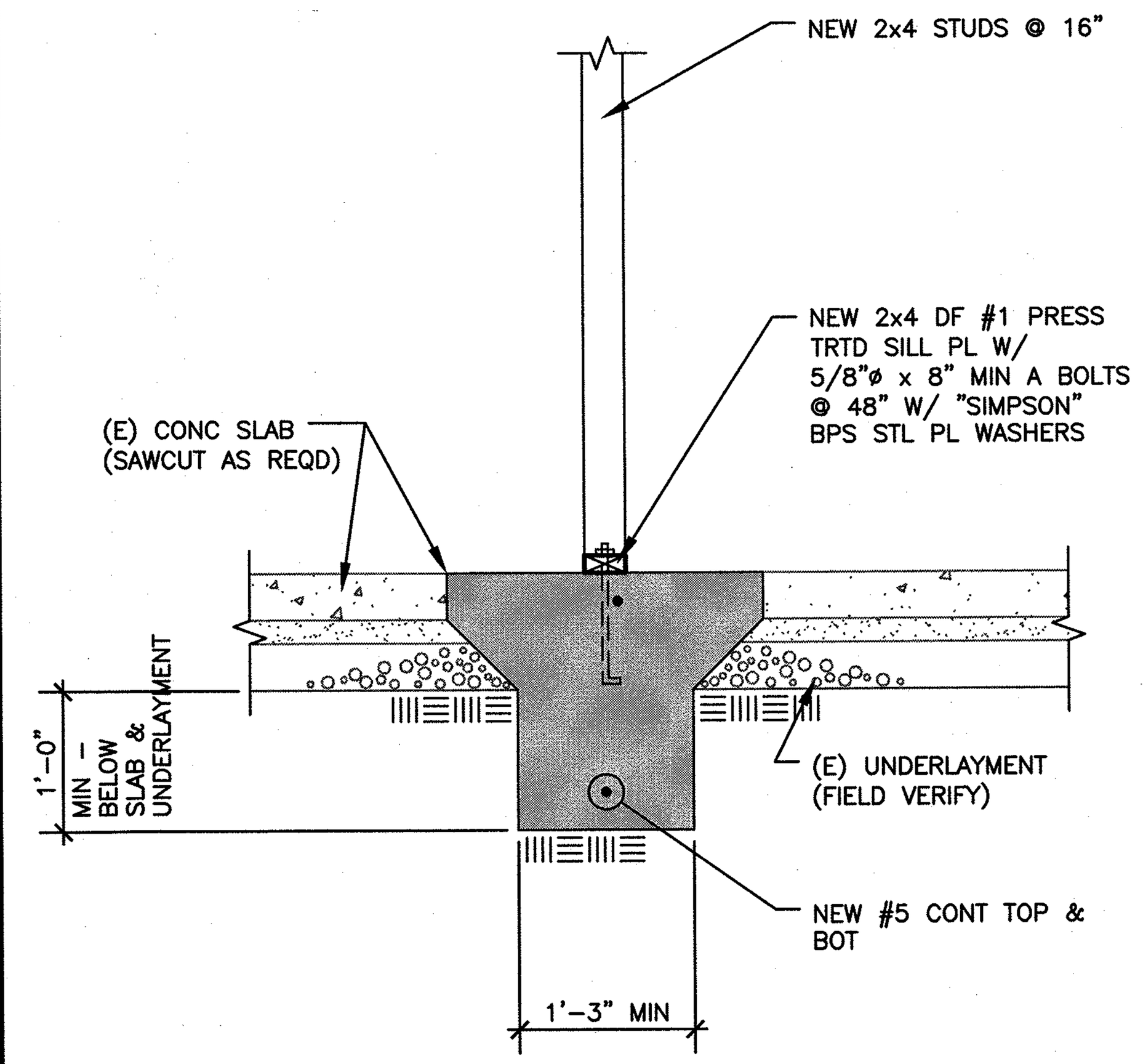
A | 1/8" | **PARTIAL ROOF FRAMING PLAN**



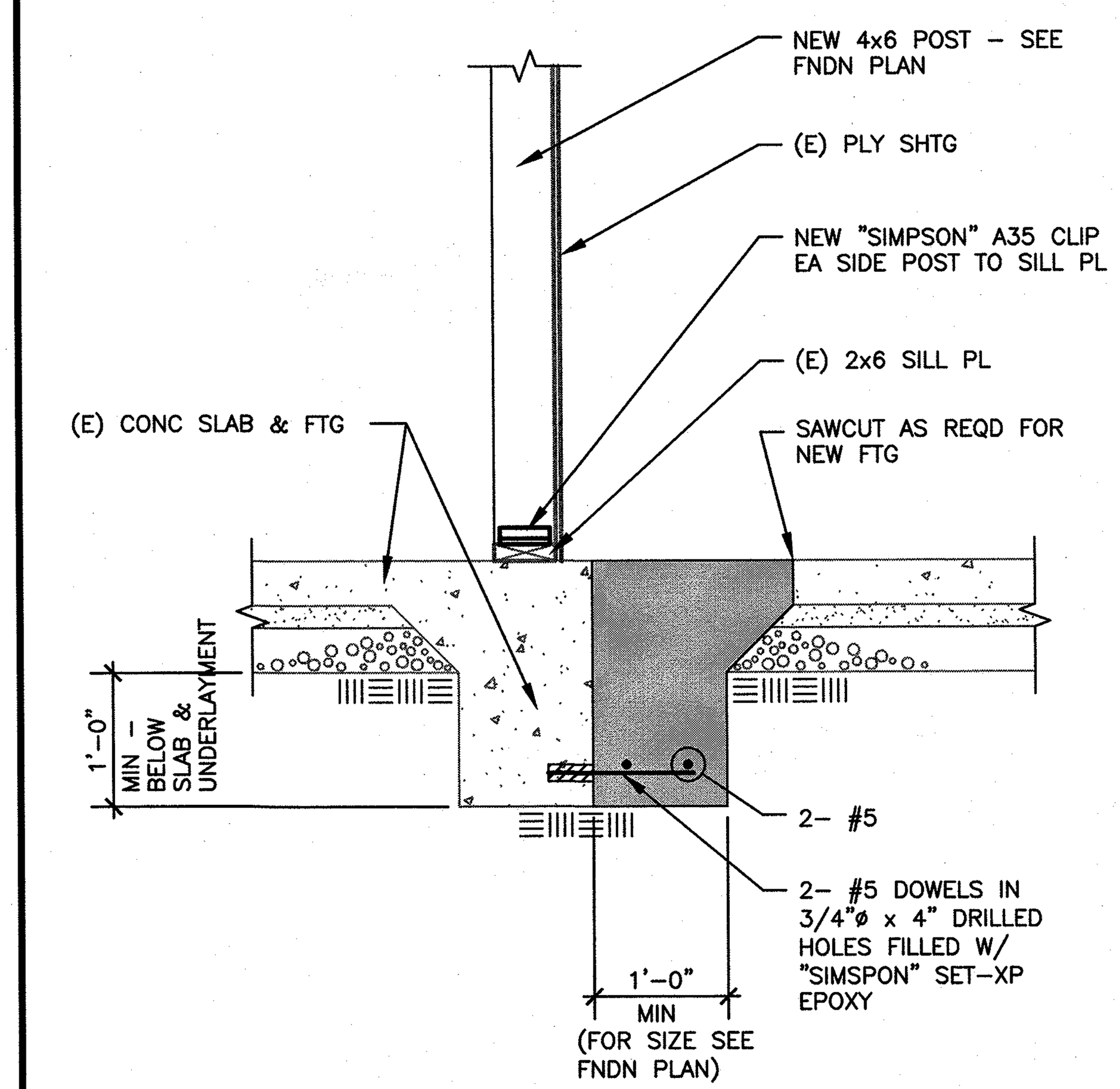
A 3/4" **SECTION AT FOUNDATION**



B 3/4" **SECTION AT FOUNDATION**



C 3/4" **SECTION AT FOUNDATION**



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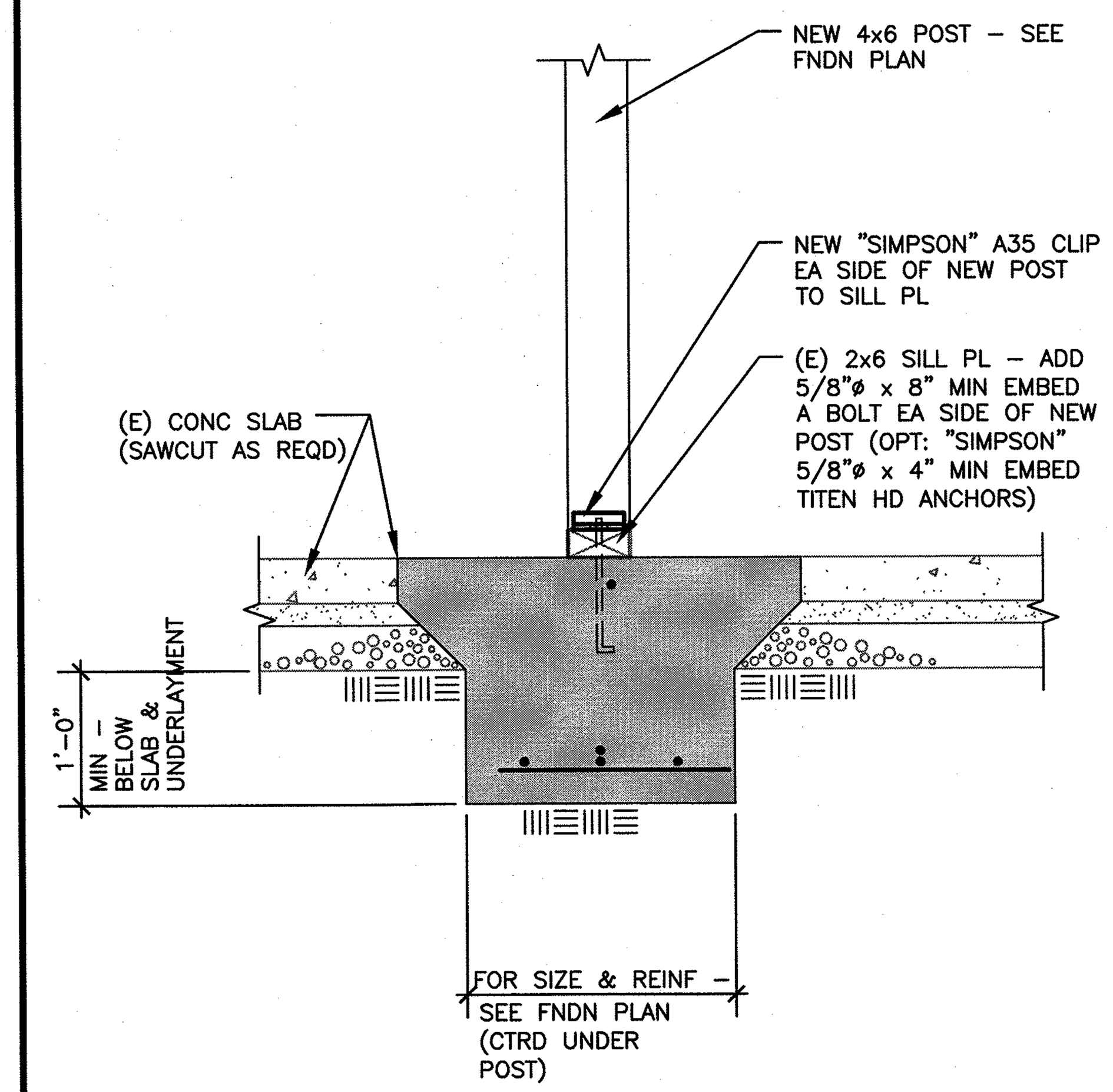

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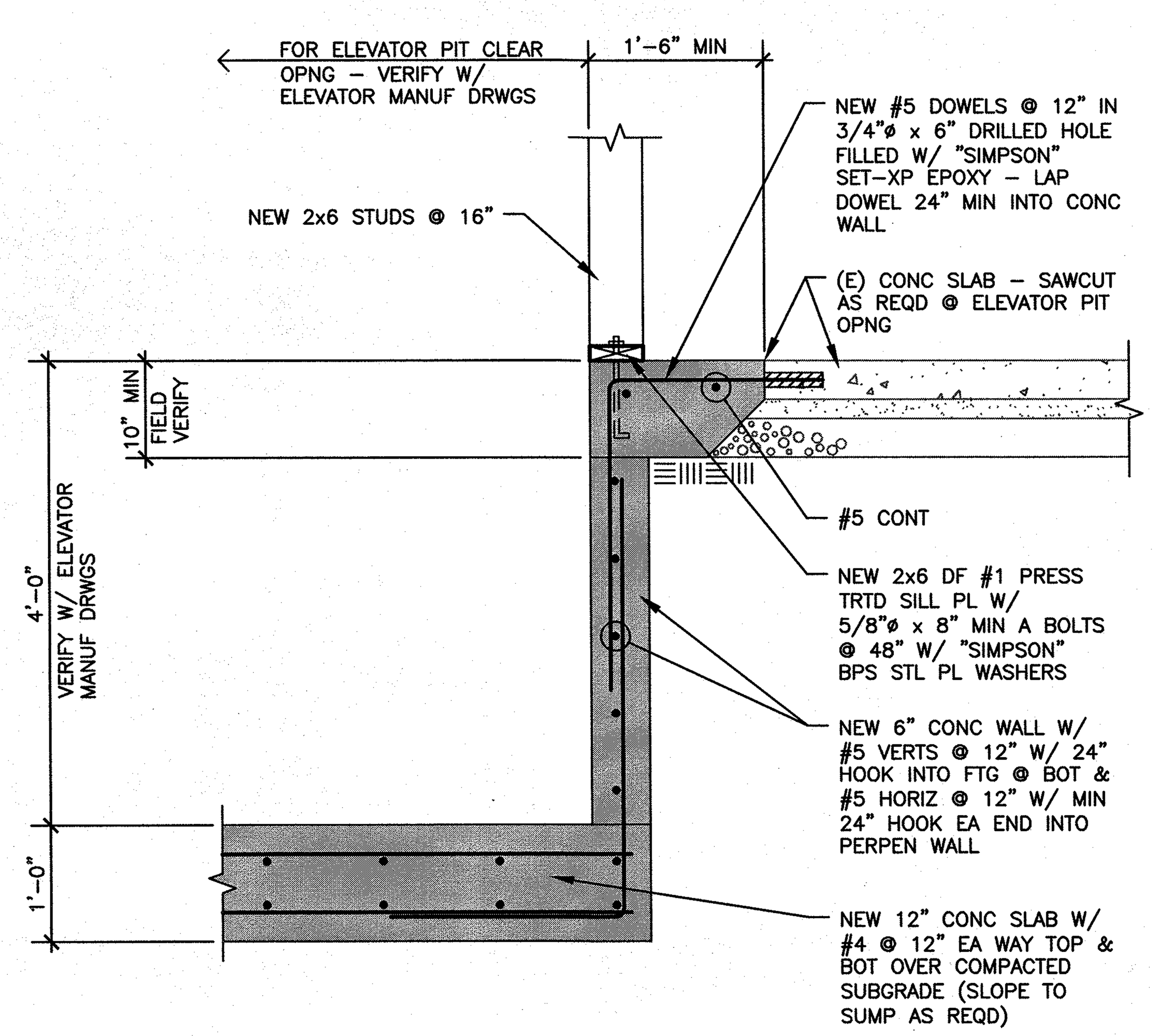
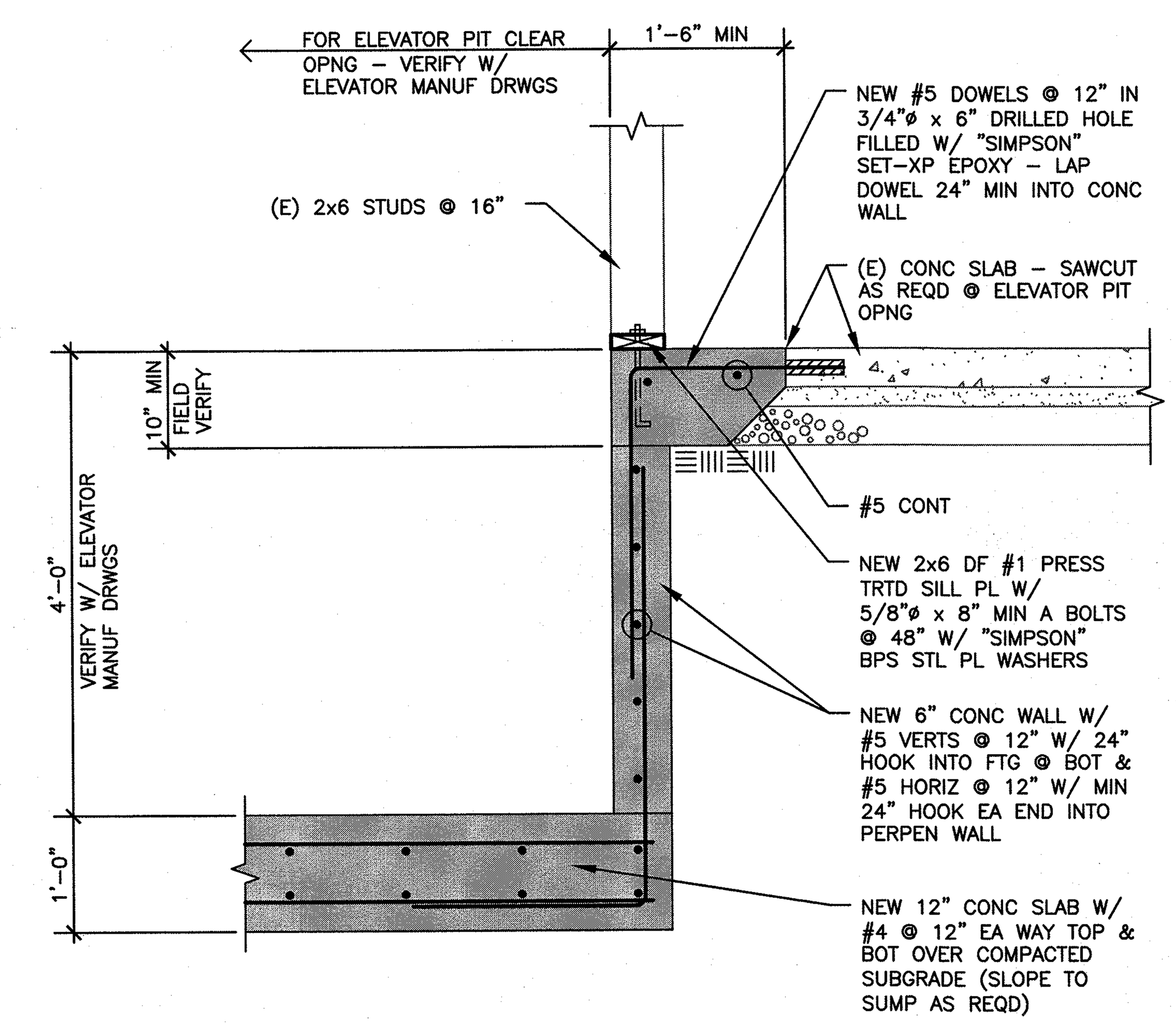
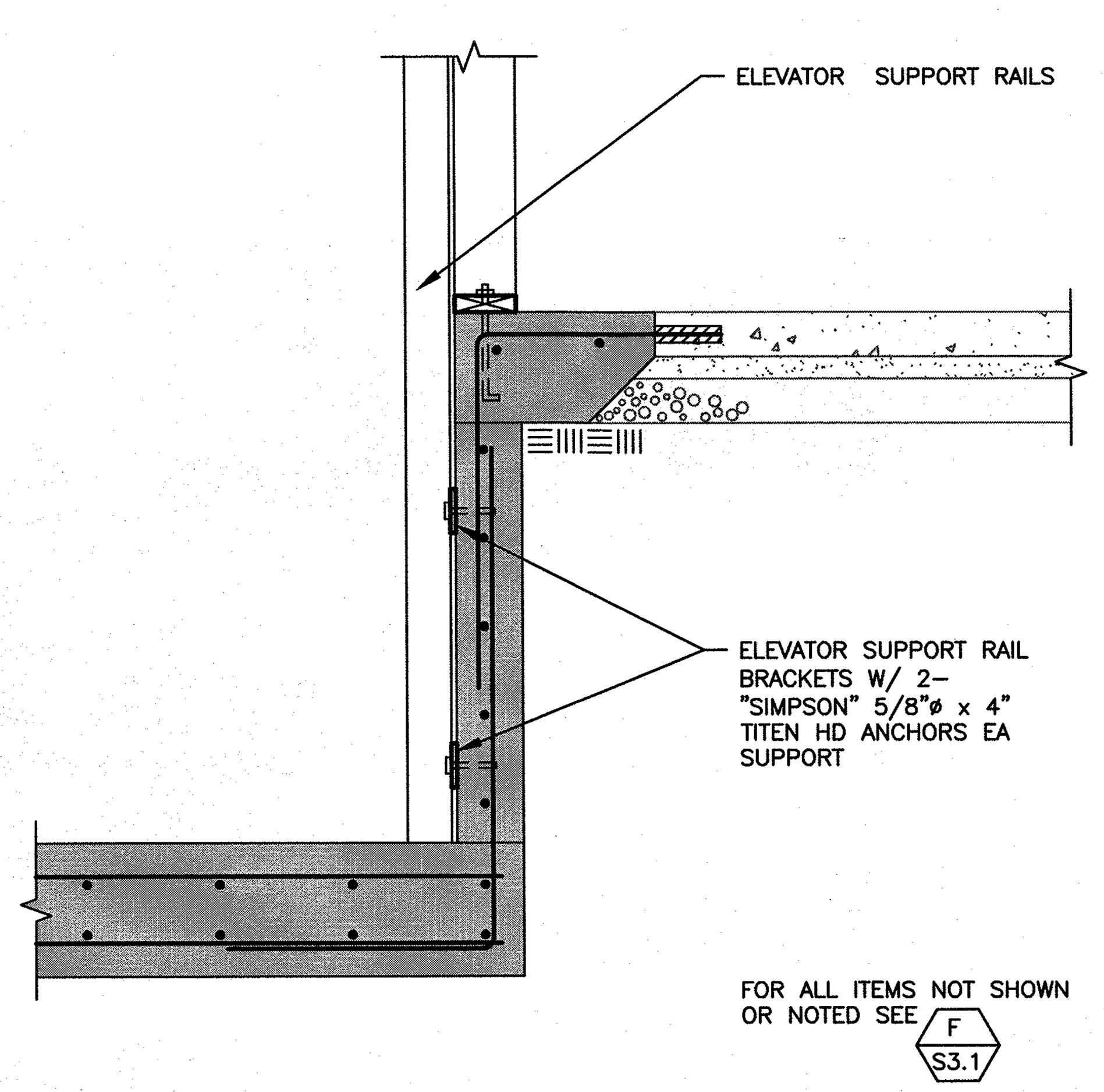


F 3/4" **SECTION AT FOUNDATION**



G 3/4" **SECTION AT FOUNDATION**



J 3/4" **SECTION AT FOUNDATION**

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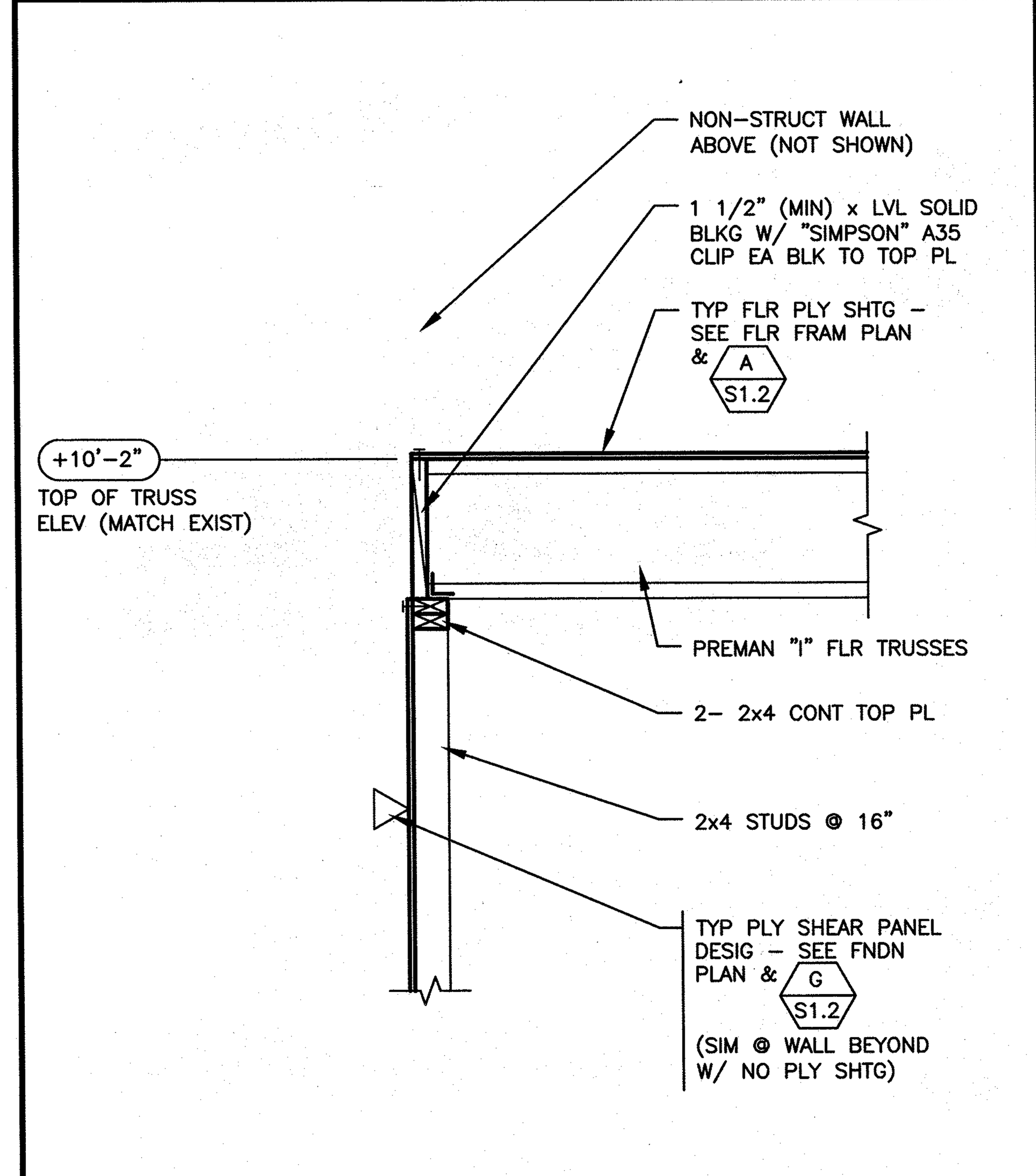
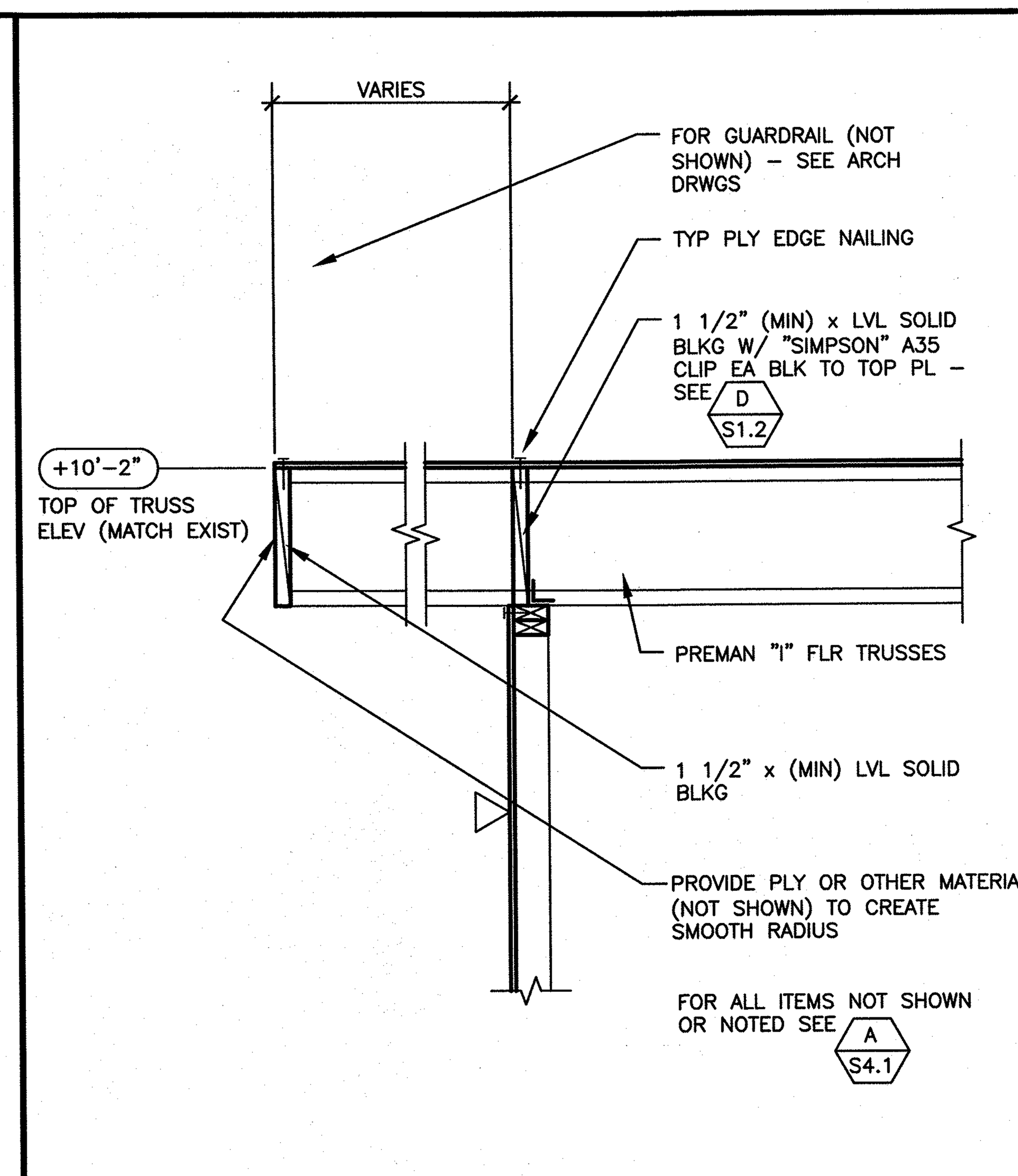
FOUNDATION DETAILS

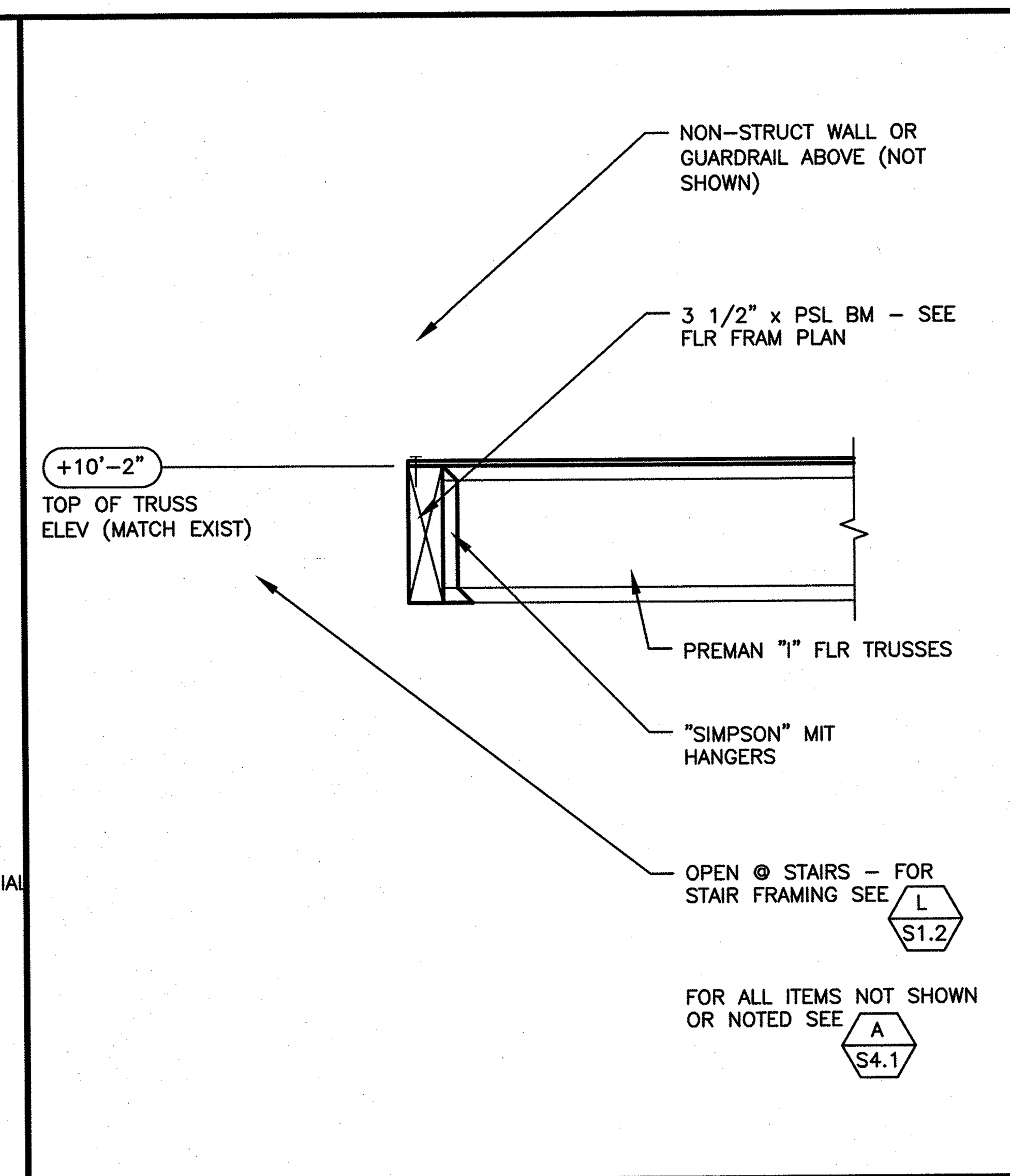
S3.1



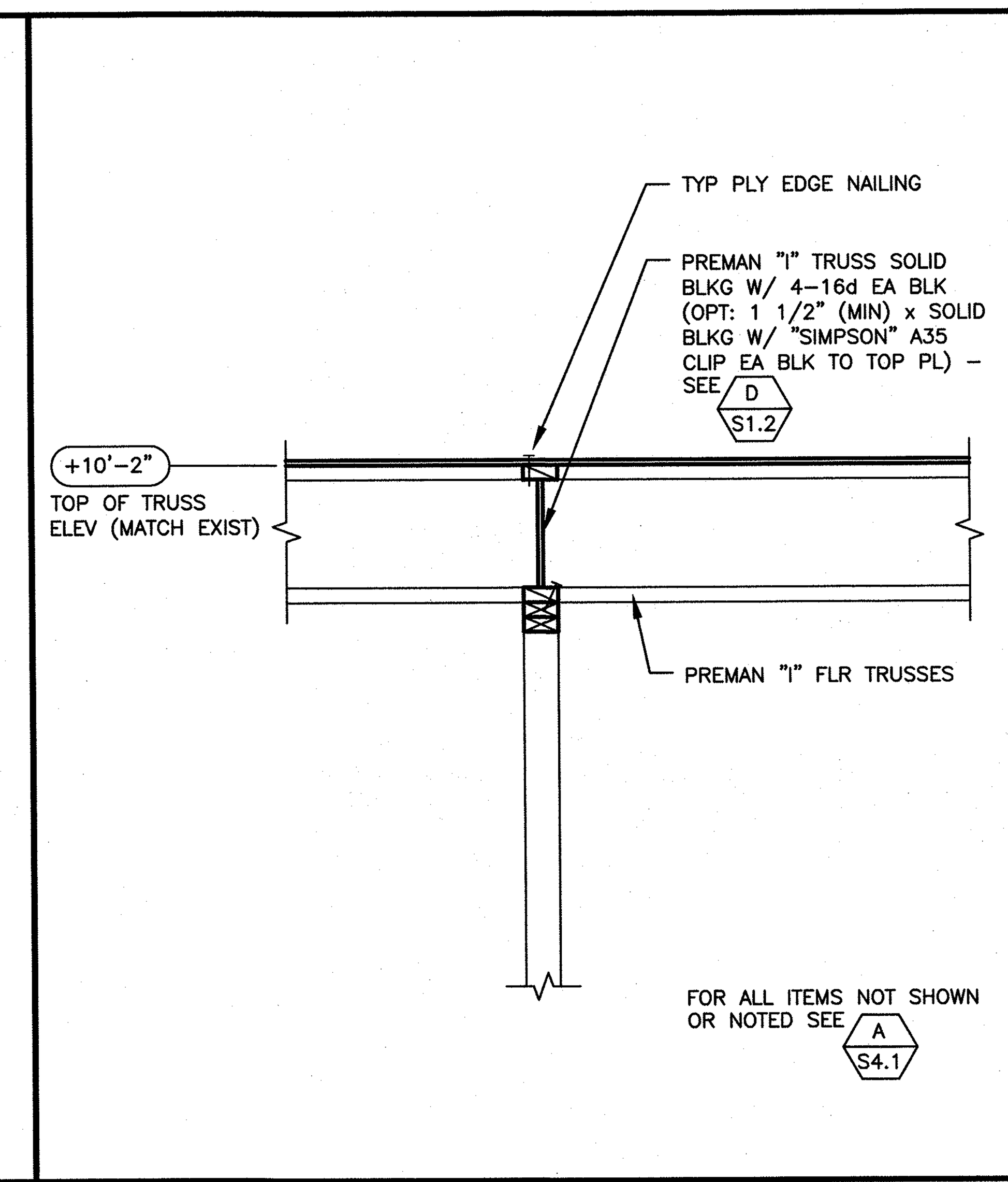
A 3/4" SECTION AT FLOOR FRAMING



B 3/4" SECTION AT FLOOR FRAMING



C 3/4" SECTION AT FLOOR FRAMING



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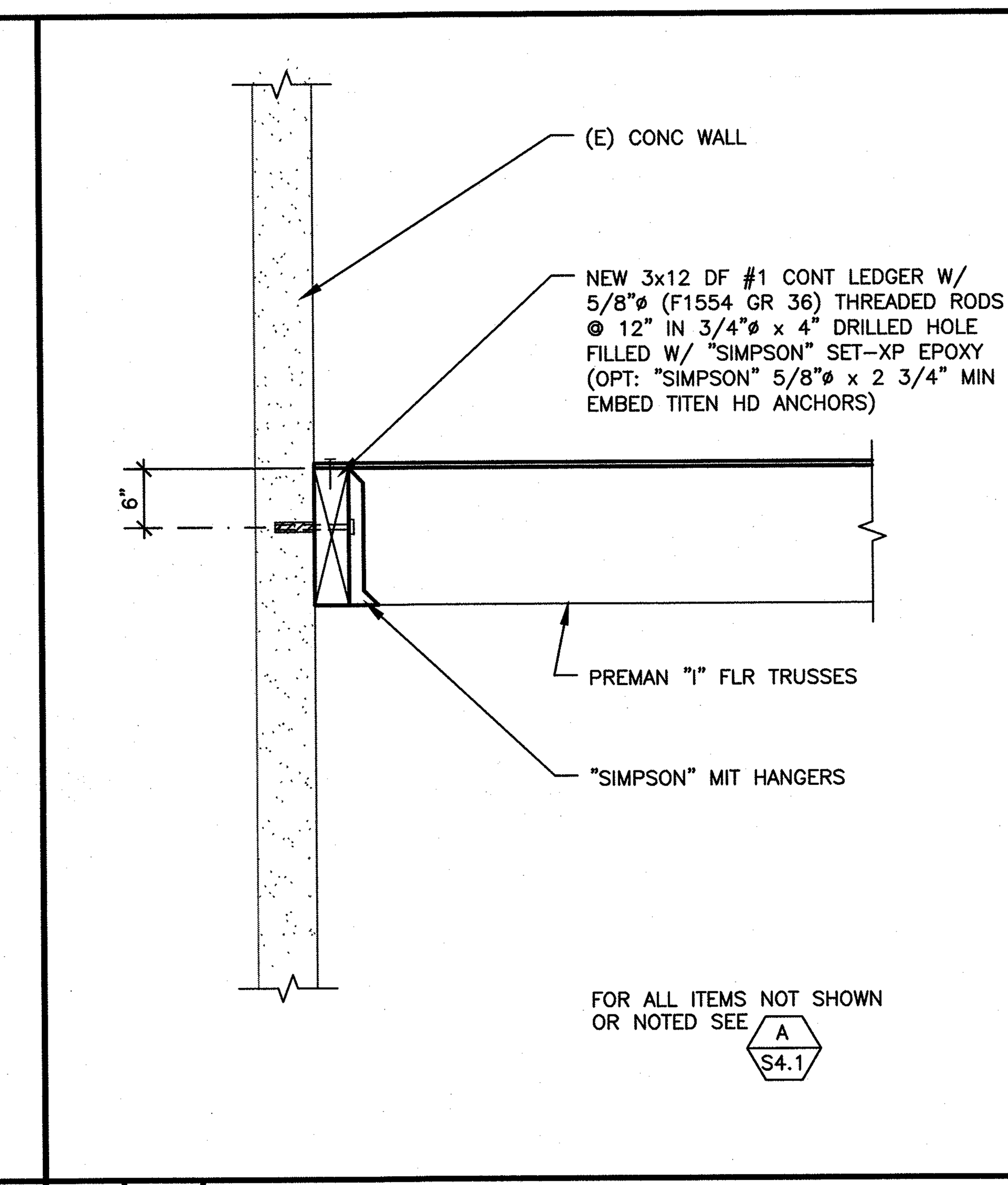
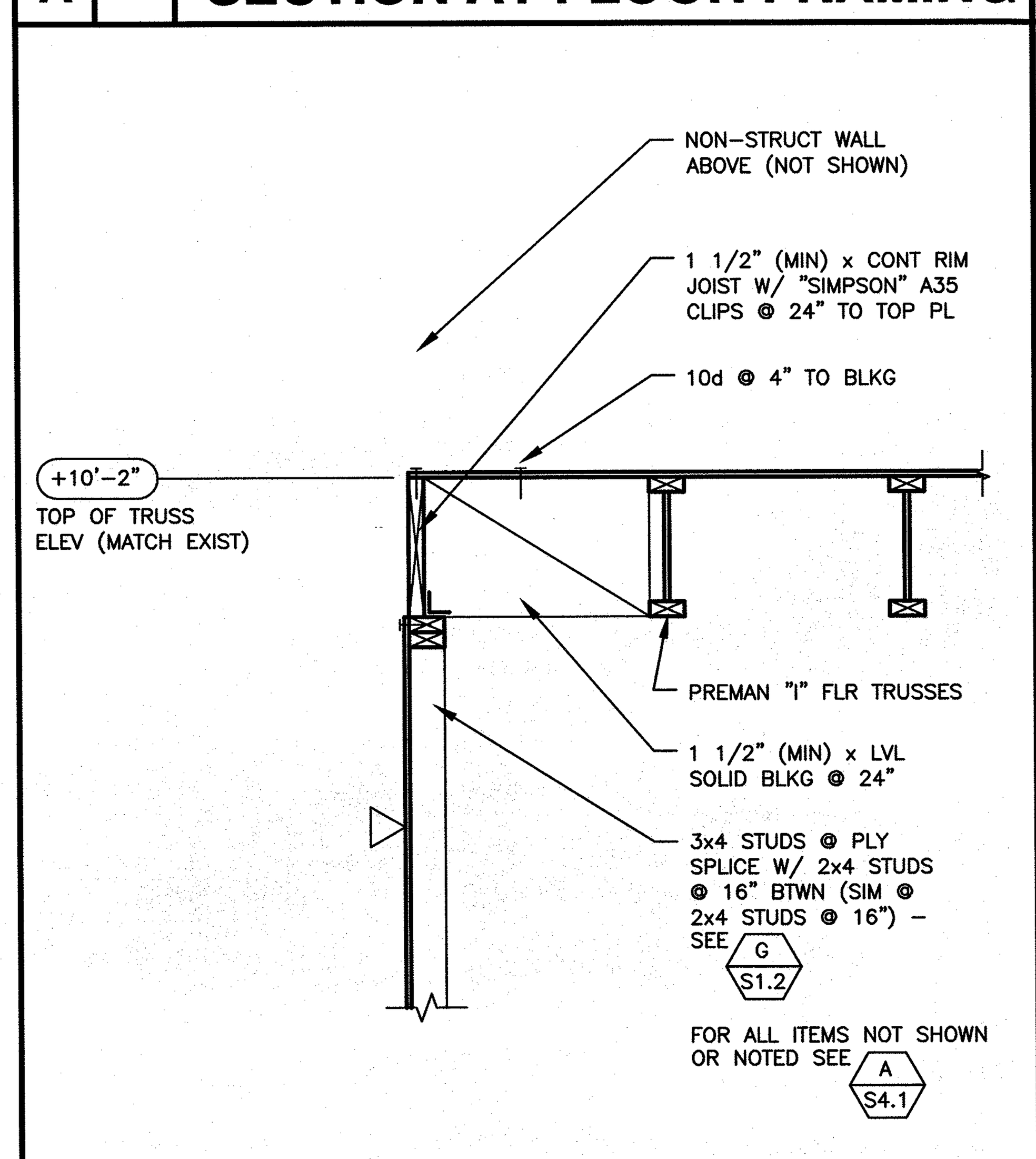
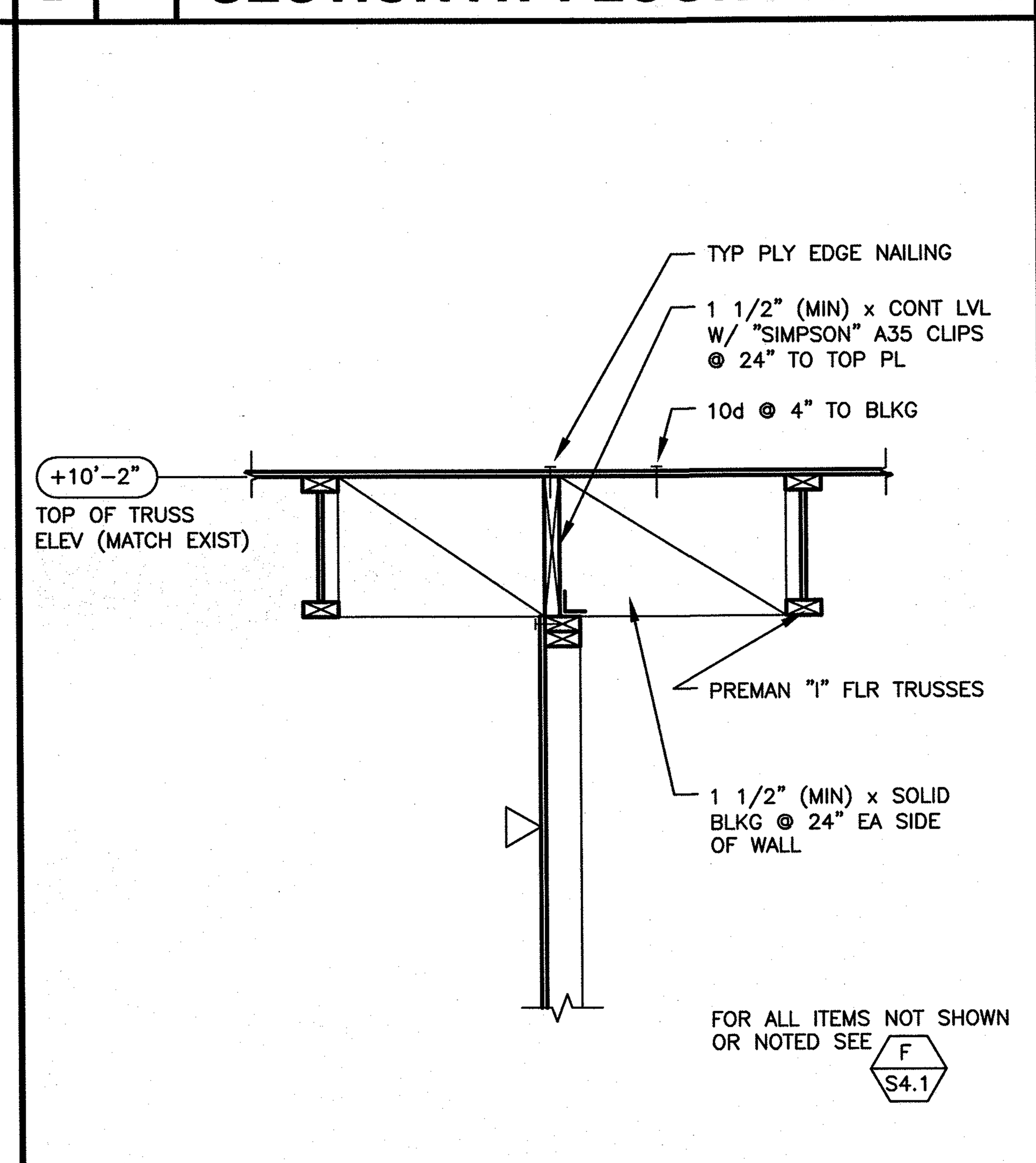


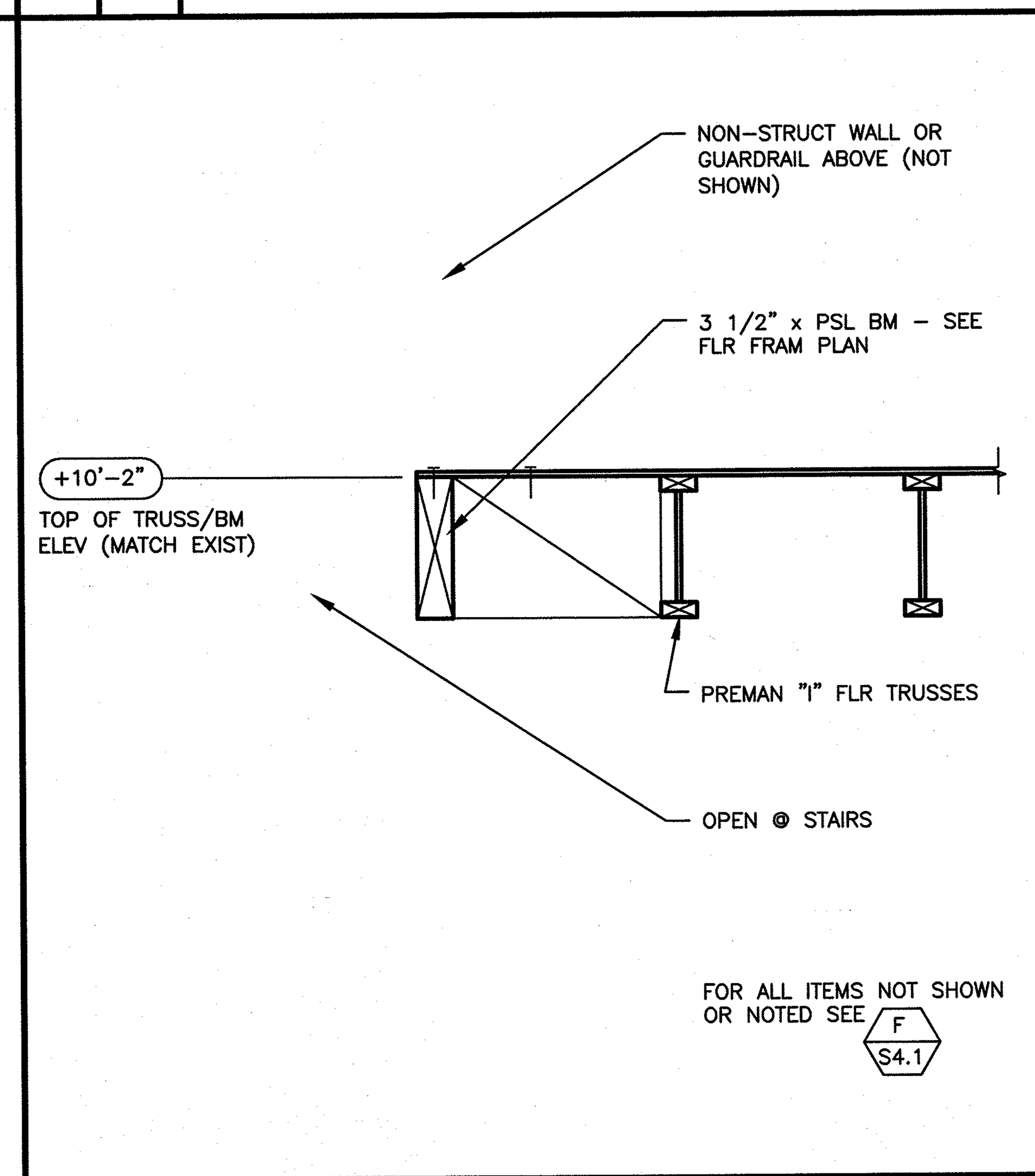
E 3/4" SECTION AT FLOOR FRAMING



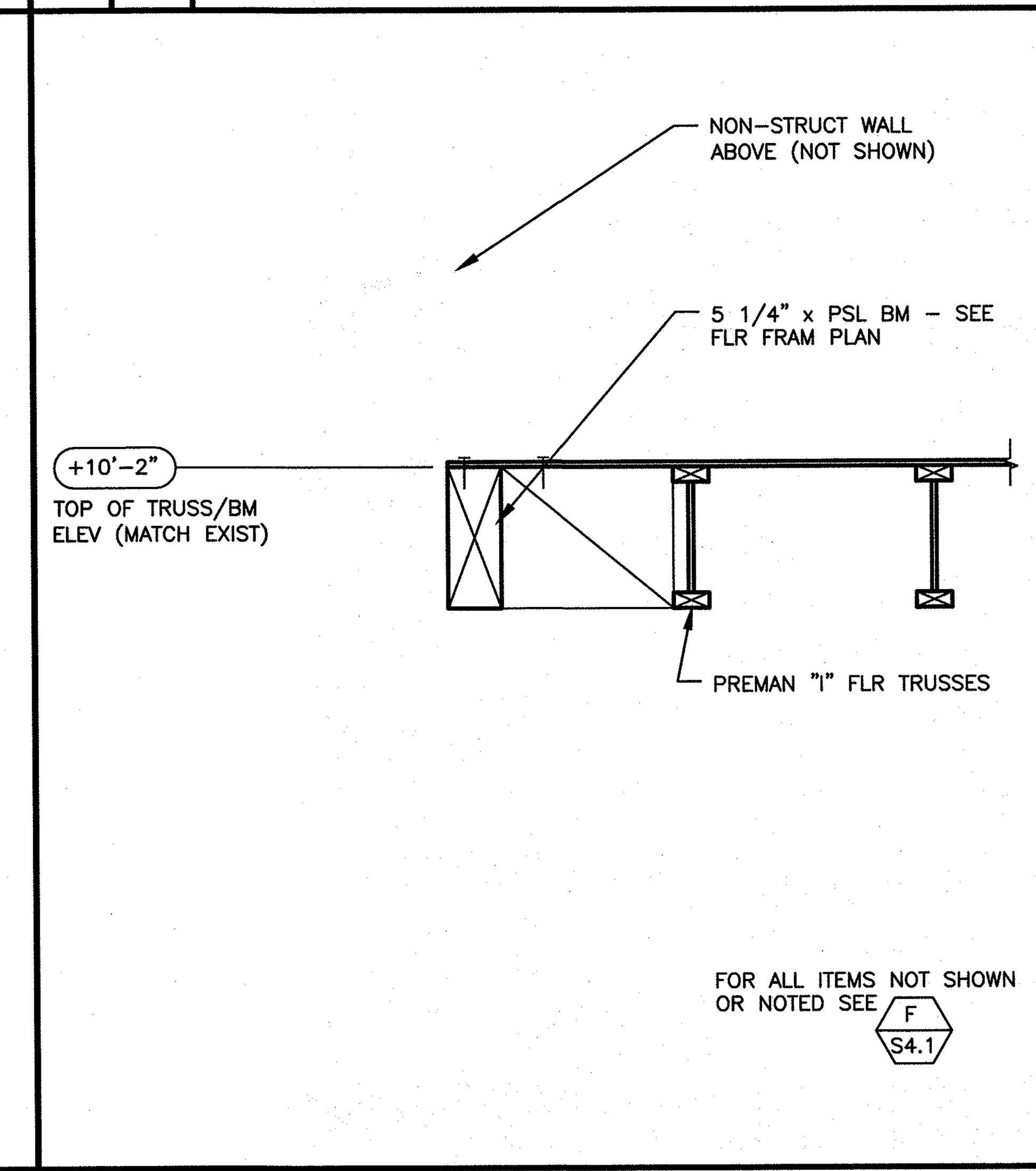
F 3/4" SECTION AT FLOOR FRAMING



G 3/4" SECTION AT FLOOR FRAMING



H 3/4" SECTION AT FLOOR FRAMING



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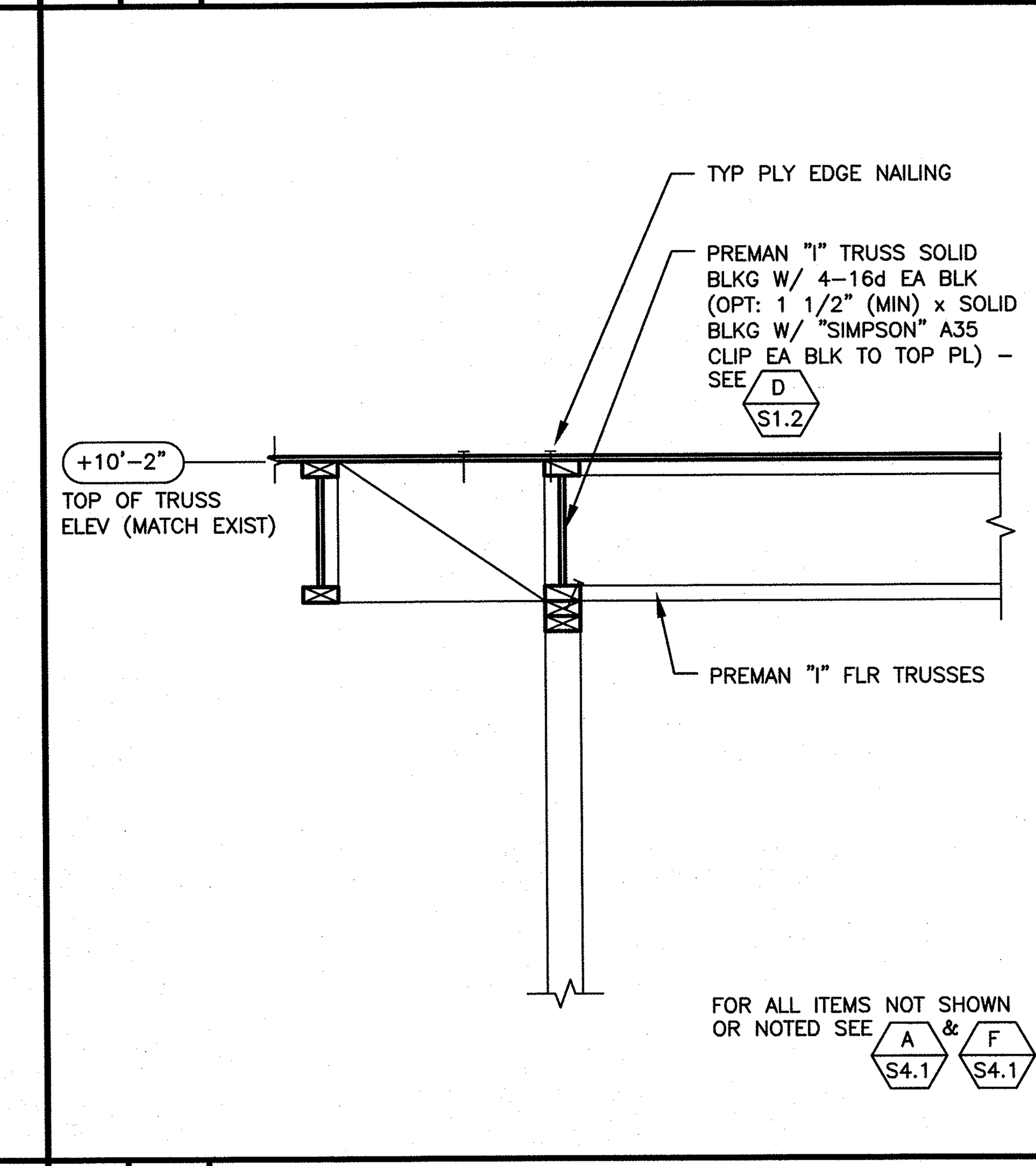
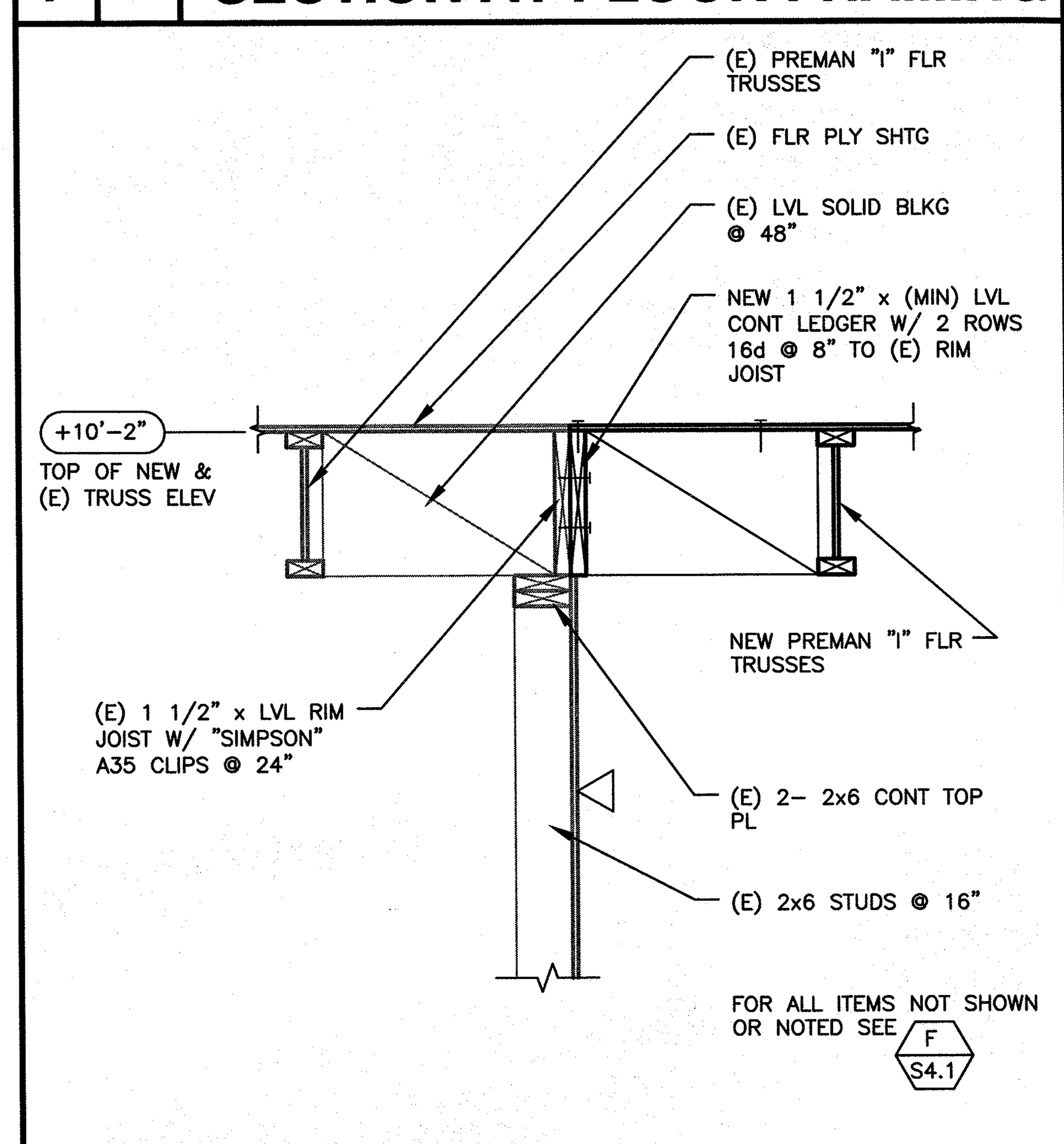
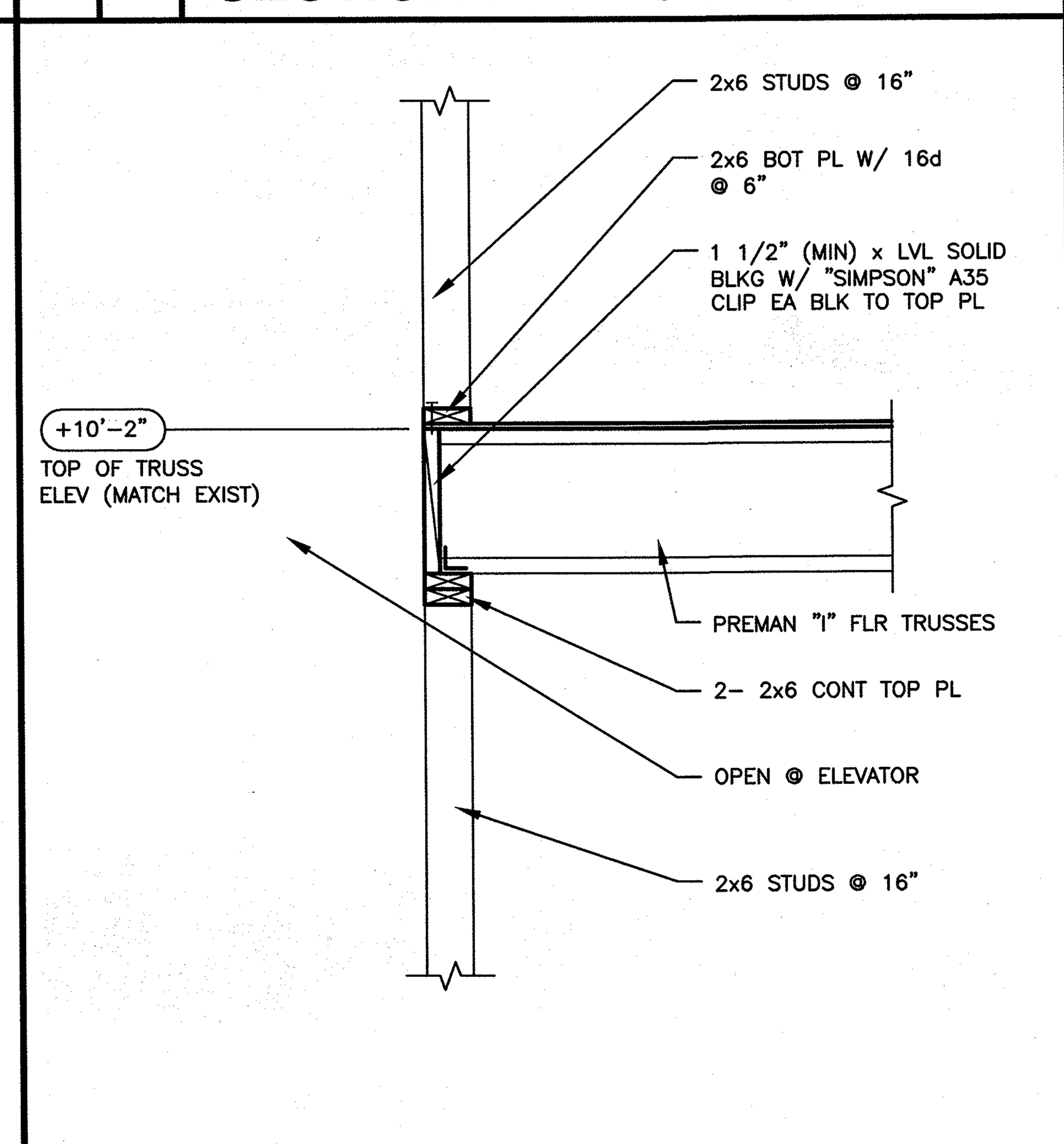


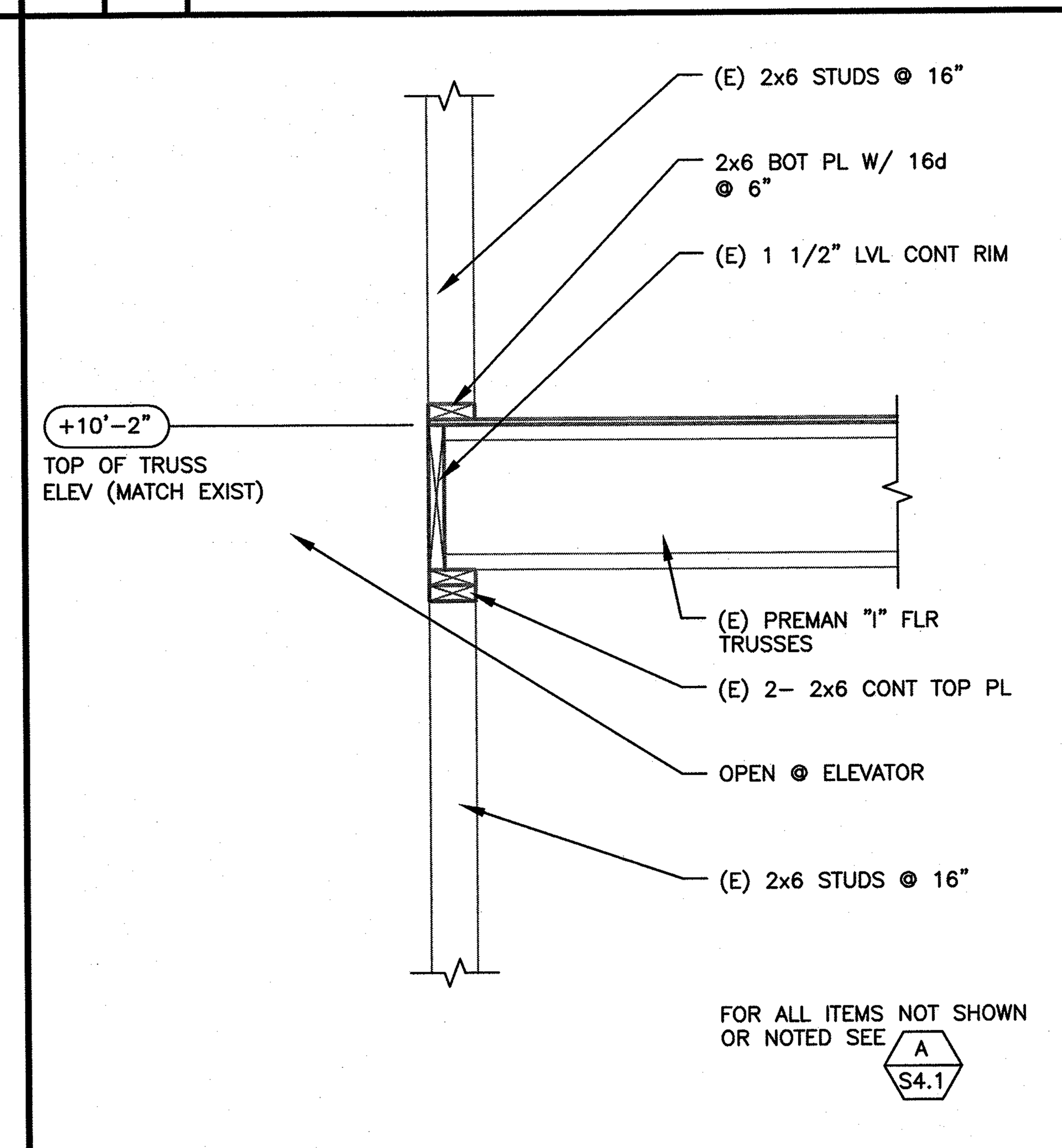
K 3/4" SECTION AT FLOOR FRAMING



L 3/4" SECTION AT FLOOR FRAMING



M 3/4" SECTION AT FLOOR FRAMING



N 3/4" SECTION AT FLOOR FRAMING

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REVIEWED FOR CODE COMPLIANCE

DEC 3 0 2014

By________ Permit No.________

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12.19.14

L Street Architects Limited
1414 L Street
Modesto, California 95354
209-575-1415 209-575-4374 fax
www.lstreetarchitects.com

PELTON WYLIE ENGINEERING, INC
2813 COFFEE ROAD SUITE D1
MODESTO, CALIFORNIA 95355
TEL: (209)575-9619 FAX: (209)575-0274

PROPOSED TENANT IMPROVEMENT FOR:

CENTER FOR HUMAN SERVICES

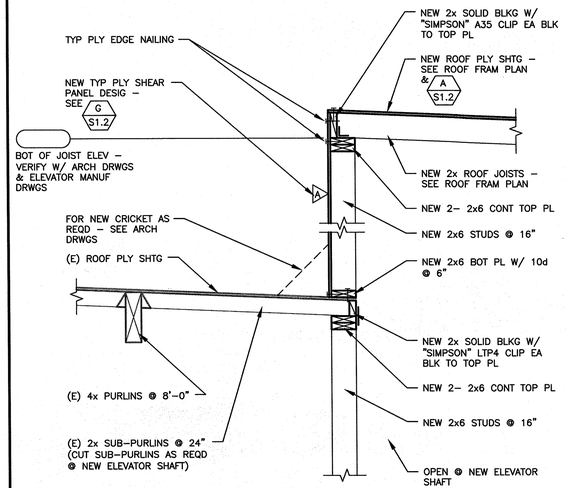
2000 W BRIGGSMORE, SUITE I
MODESTO, CALIFORNIA 95350

REVISIONS:

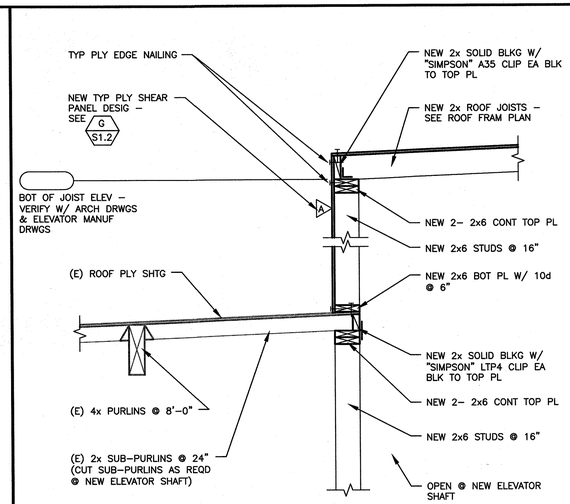
DRAWN:	SG
DATE:	OCT 2014
FILE:	
JOB:	LSA 015-14
PRINT DATE:	

FLOOR FRAMING DETAILS

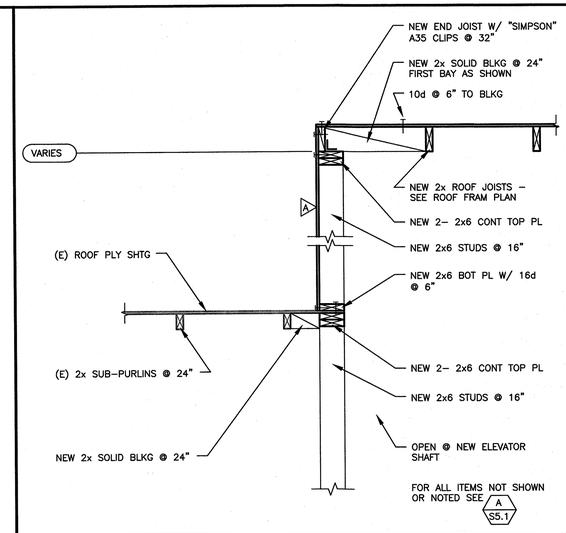
S4.1



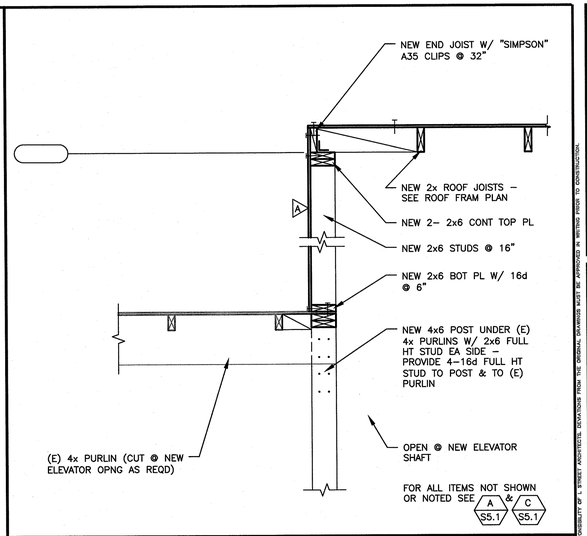
A 3/4" **SECTION AT ROOF FRAMING**



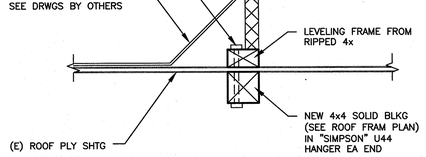
B 3/4" **SECTION AT ROOF FRAMING**



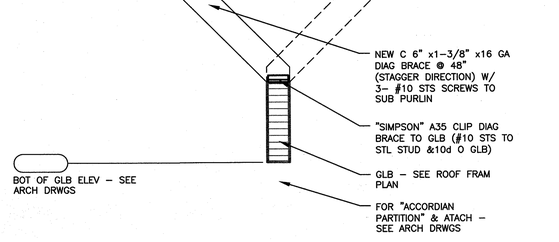
C 3/4" **SECTION AT ROOF FRAMING**



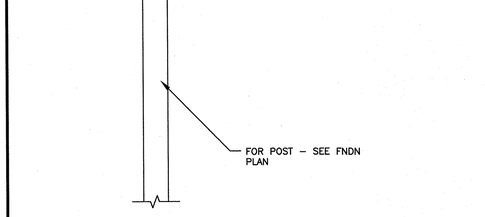
D 3/4" **SECTION AT ROOF FRAMING**



E 1 1/2" **SECTION AT ROOF FRAMING**



F 3/4" **SECTION AT "ACCORDIAN PARTITION"**



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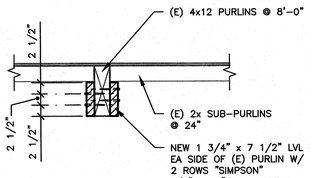
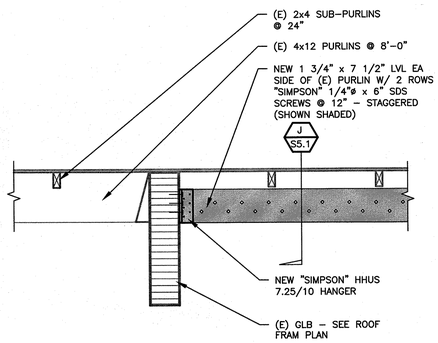
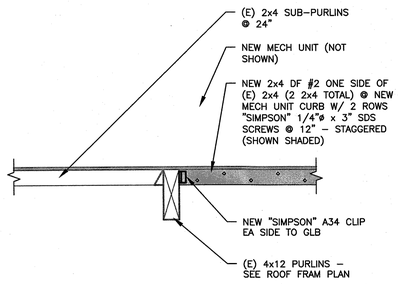


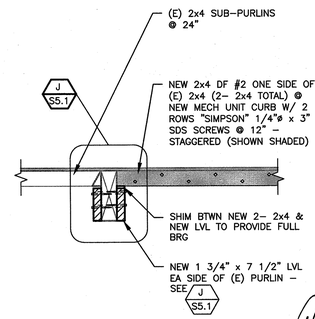
J 3/4" **SECTION AT ROOF FRAMING**



K 3/4" **SECTION AT ROOF FRAMING**



L 3/4" **SECTION AT ROOF FRAMING**



M 3/4" **SECTION AT ROOF FRAMING**

L Street Architects
Limited
1414 L Street
Modesto, California 95354
209-575-1415 209-575-4574 fax
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2813 COFFEE ROAD SUITE D1
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PROPOSED TENANT IMPROVEMENT FOR:

CENTER FOR HUMAN SERVICES

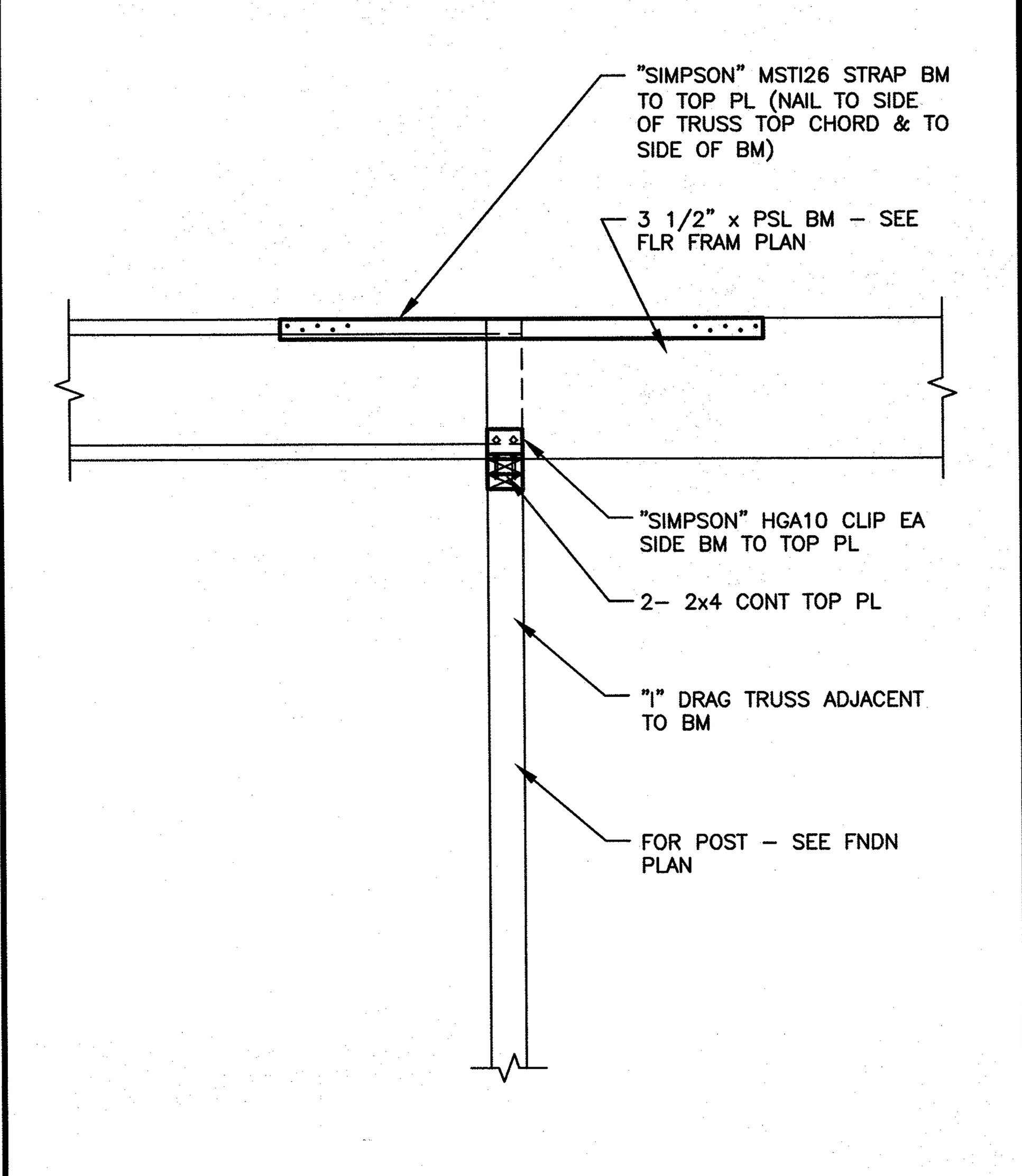
2000 W. BRIGGSMORE, SUITE I
MODESTO, CALIFORNIA 95350

REVISIONS:

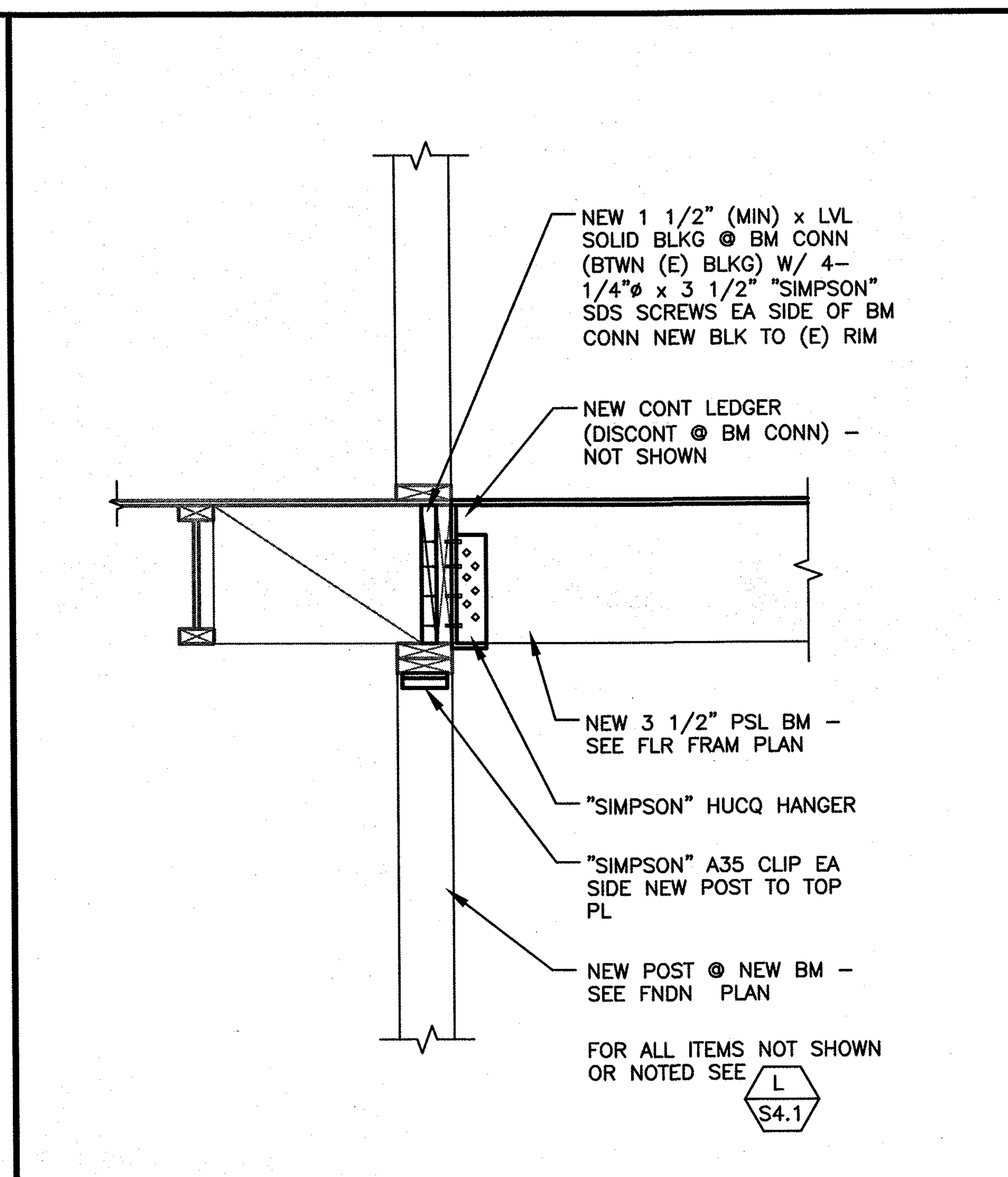
DRAWN: SG
DATE: OCT 2014
FILE:
JOB: LSA 015–14
PRINT DATE:

ROOF FRAMING DETAILS

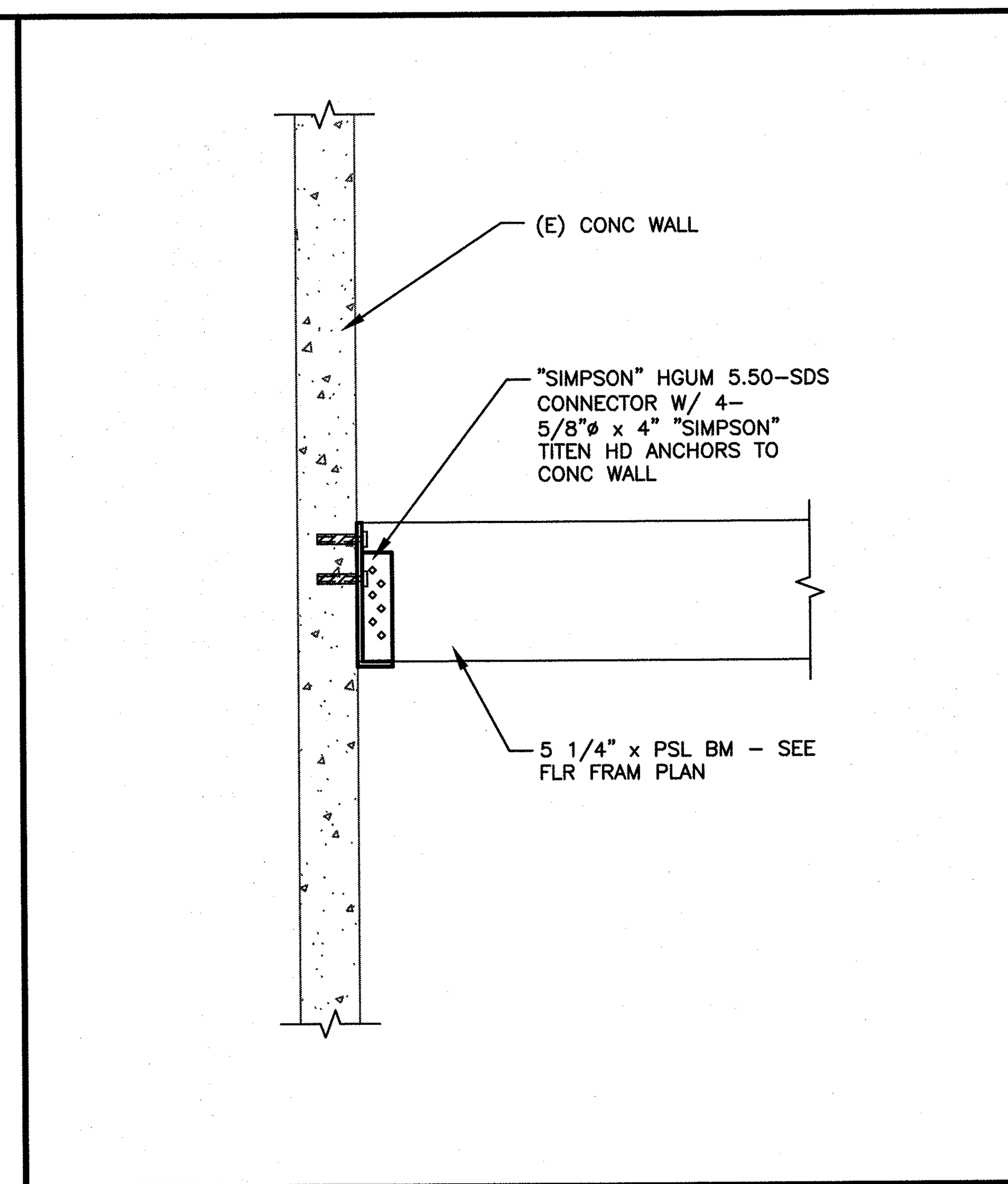
S5.1



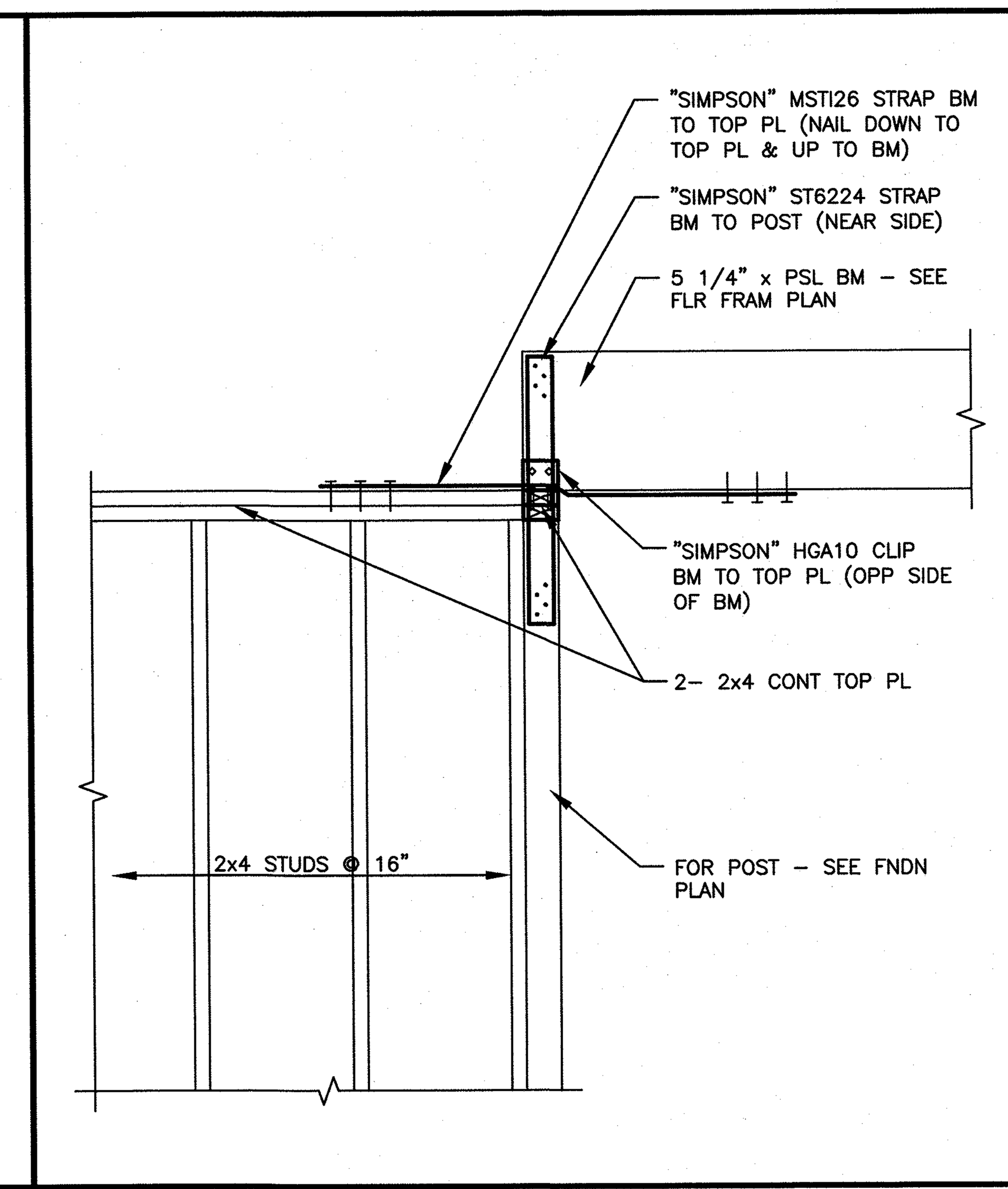
A 3/4" **SECTION AT BM CONNECTION**



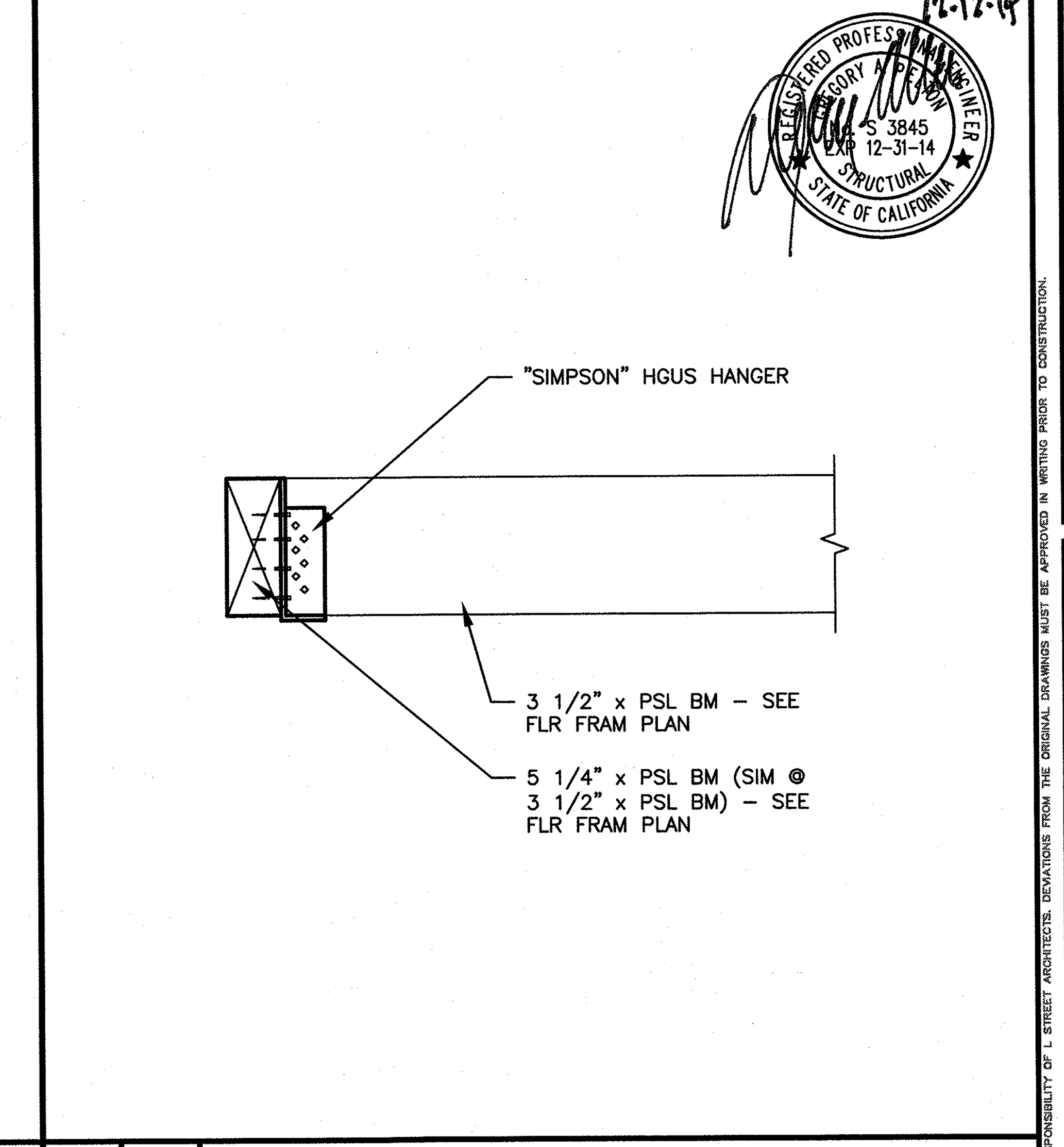
B 3/4" **SECTION AT BM CONNECTION**



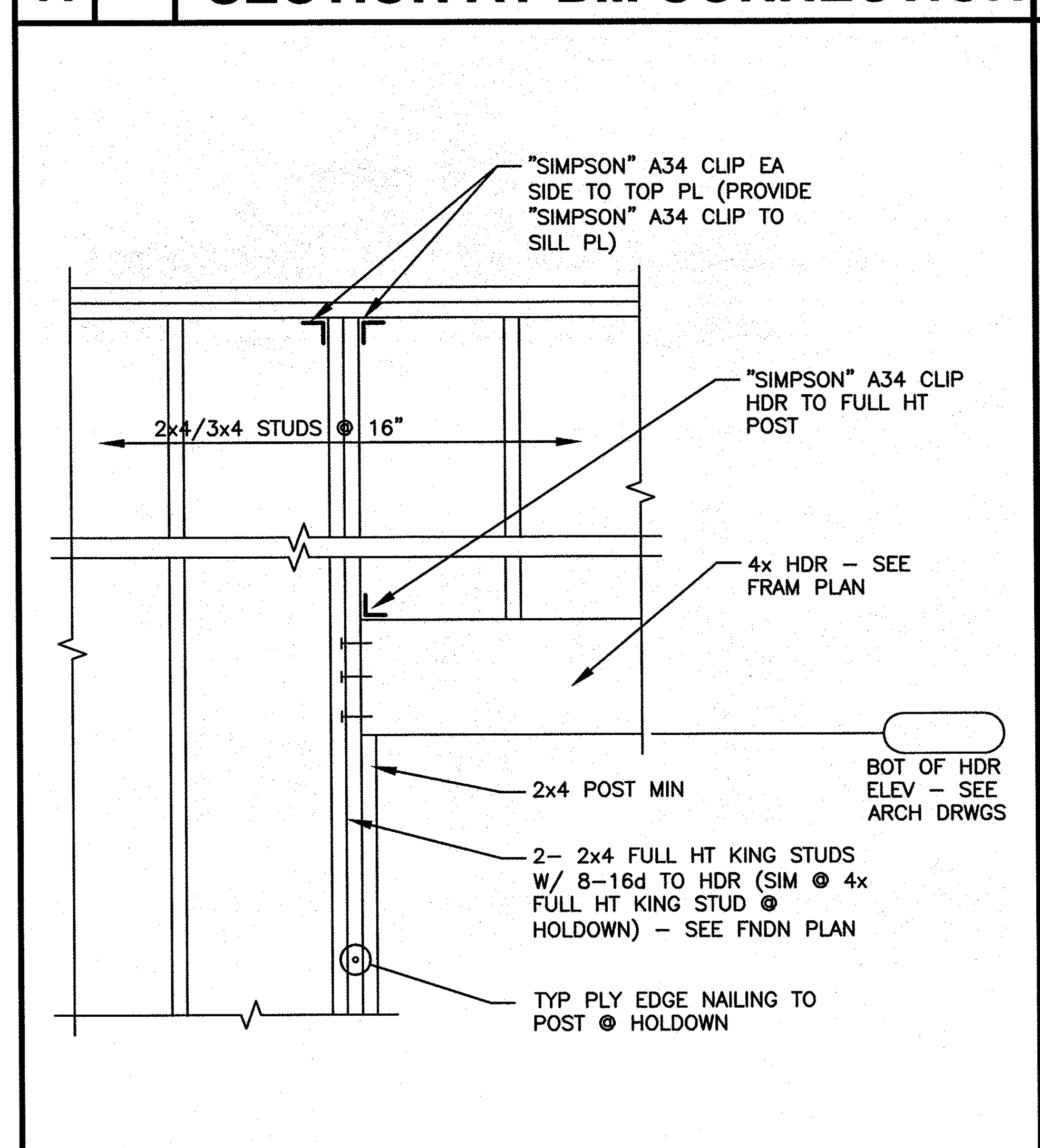
C 3/4" **SECTION AT BM CONNECTION**



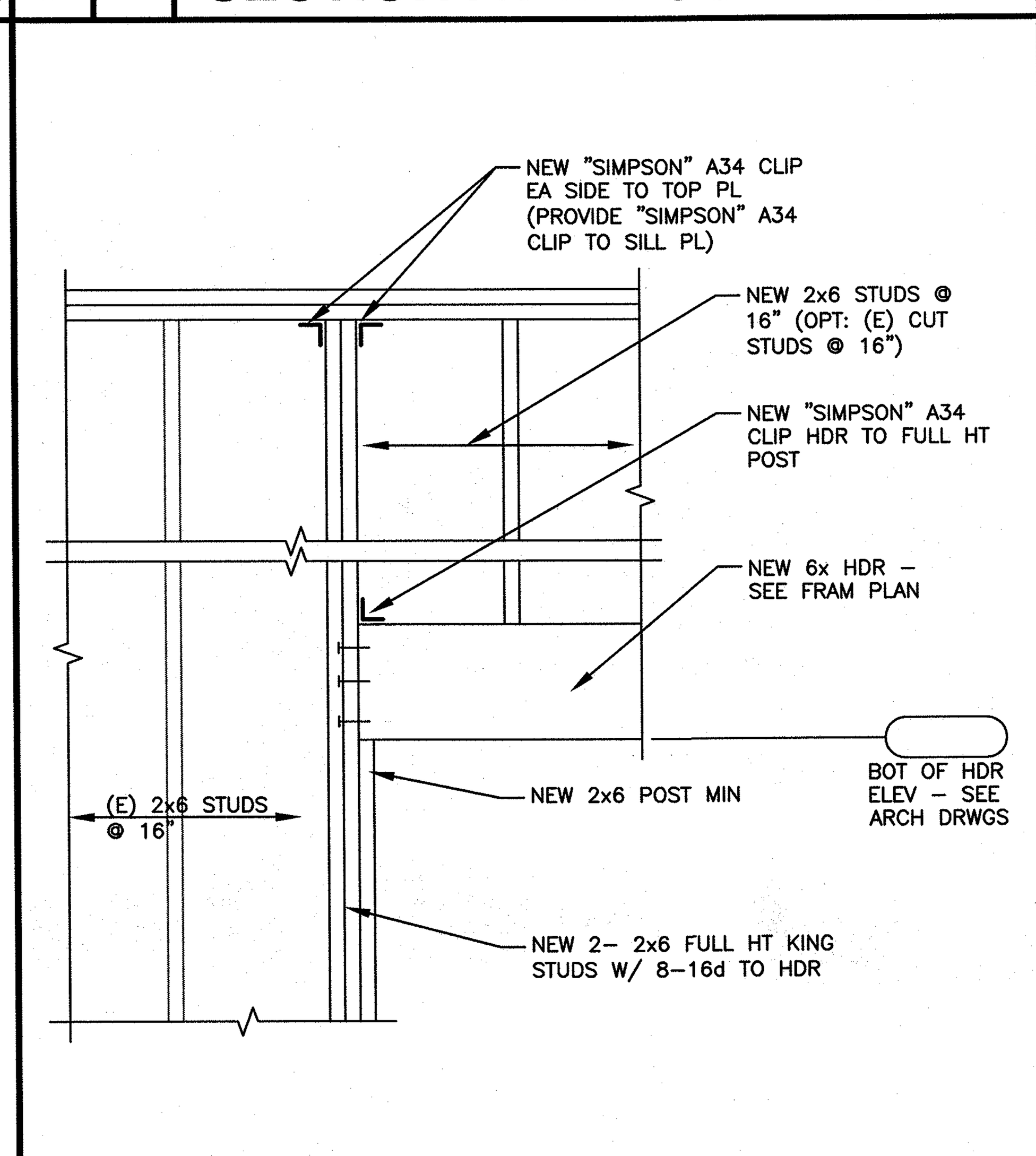
D 3/4" **SECTION AT BM CONNECTION**



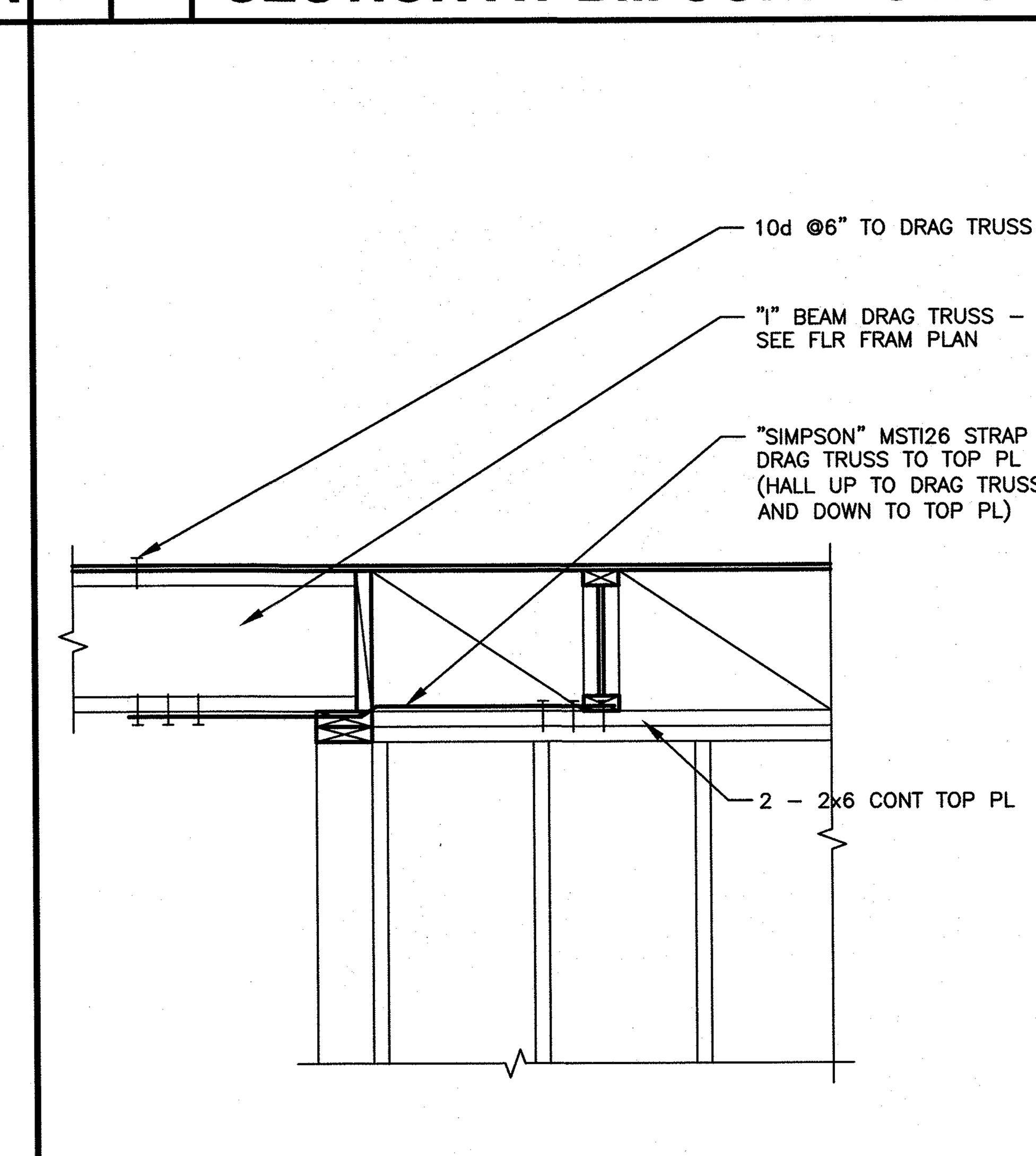
E 3/4" **SECTION AT BM CONNECTION**



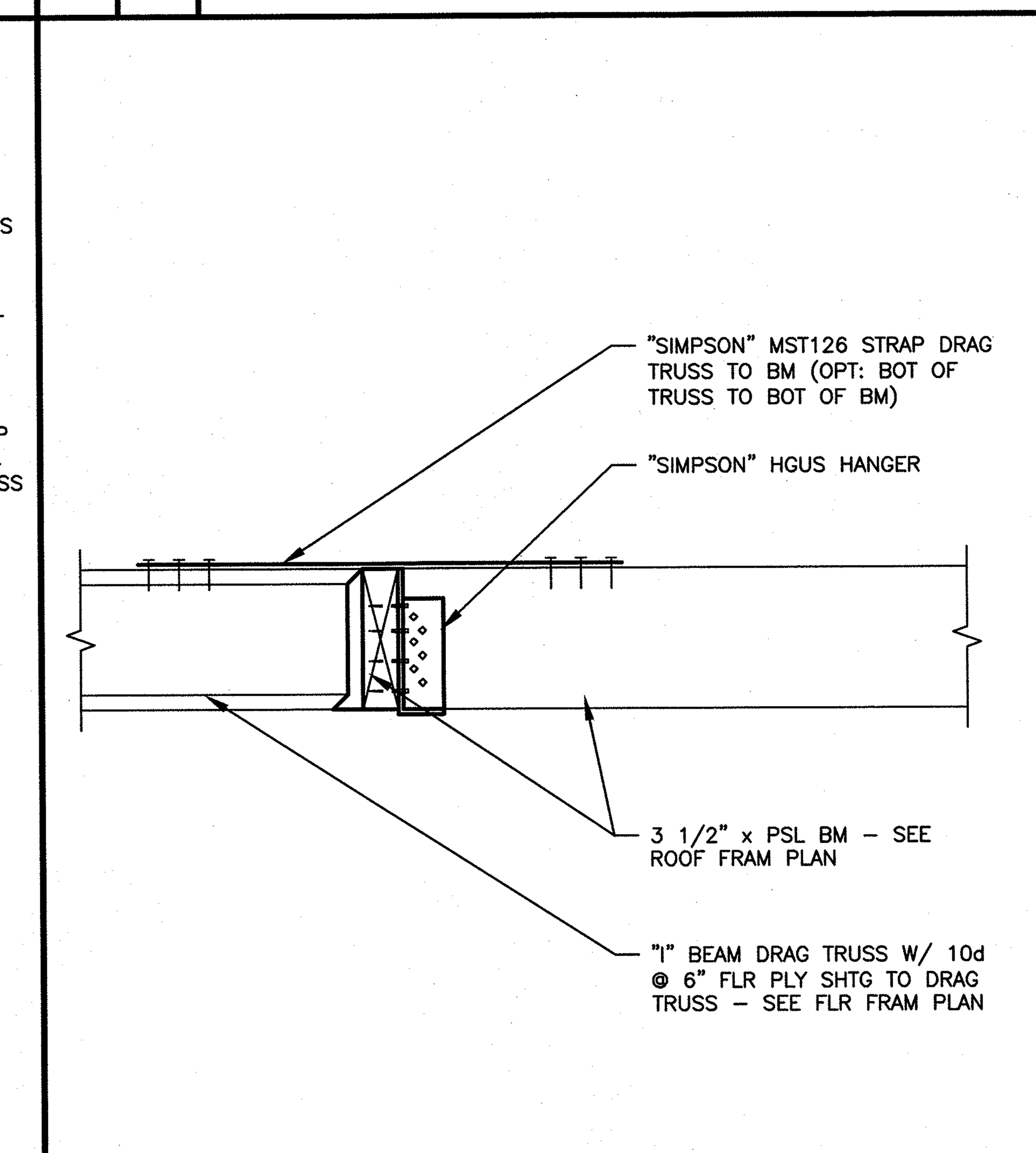
F 3/4" **SECTION AT HDR CONN**



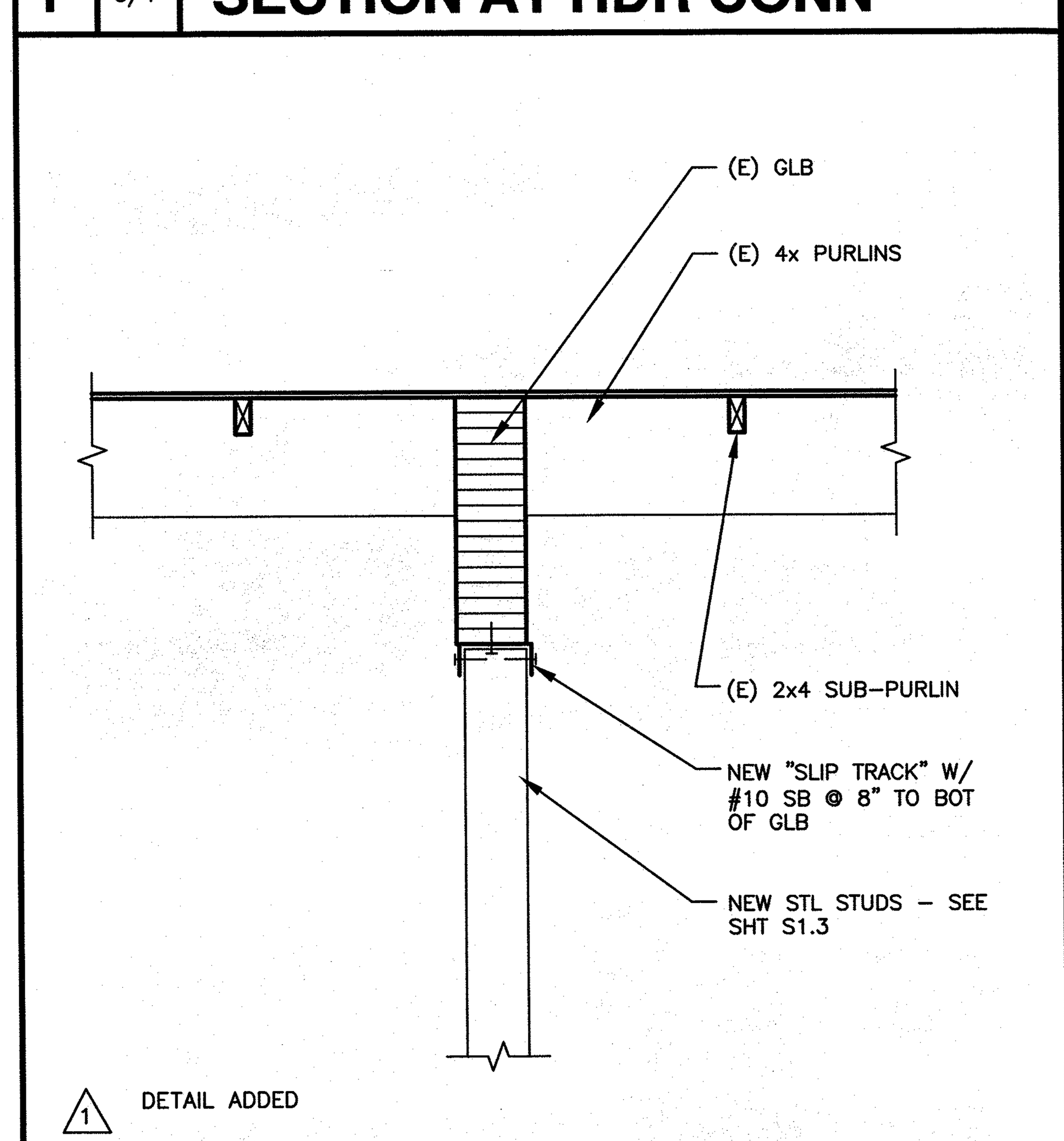
G 3/4" **SECTION AT HDR CONN**



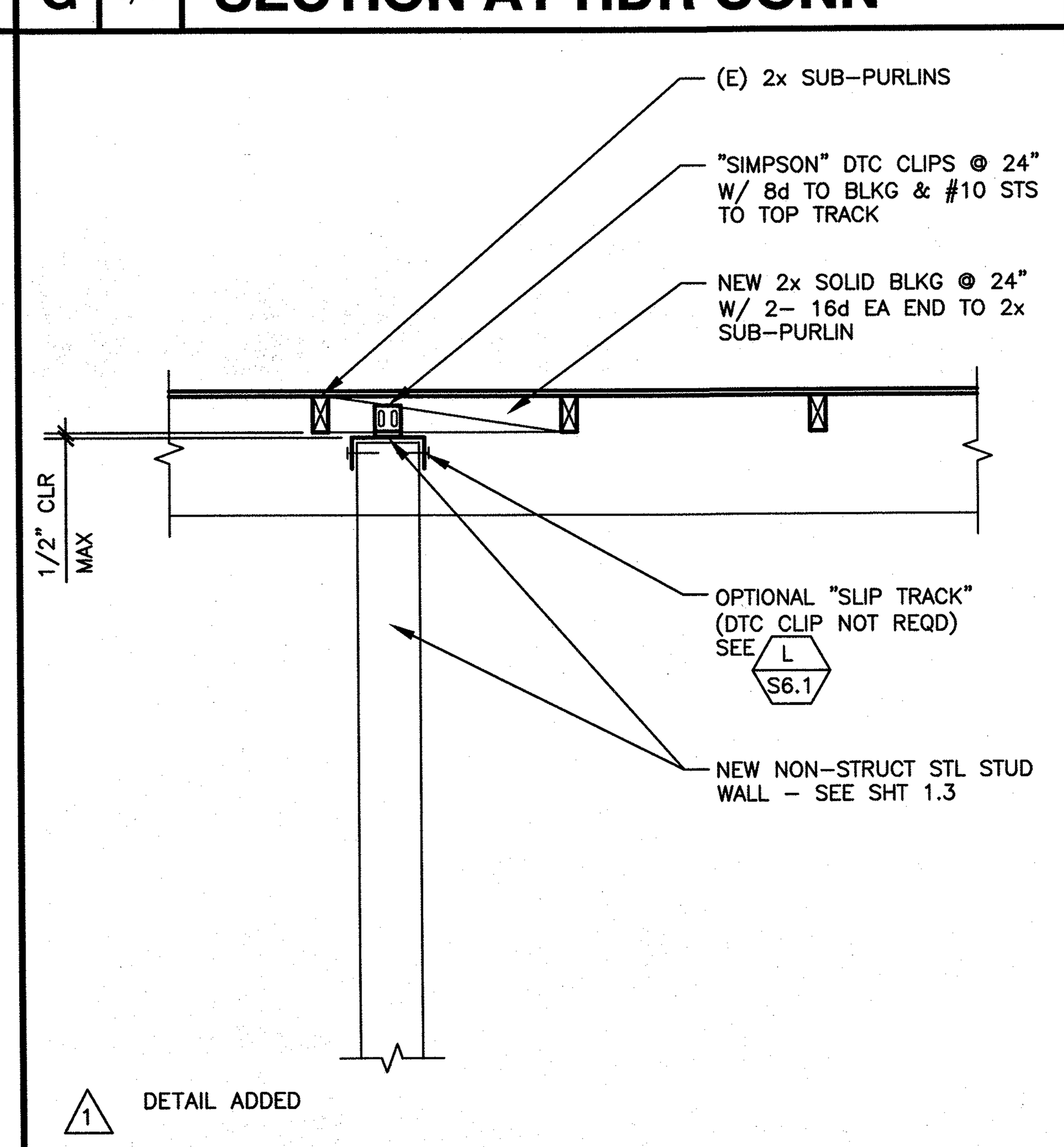
J 3/4" **SECTION AT DRAG CONN**



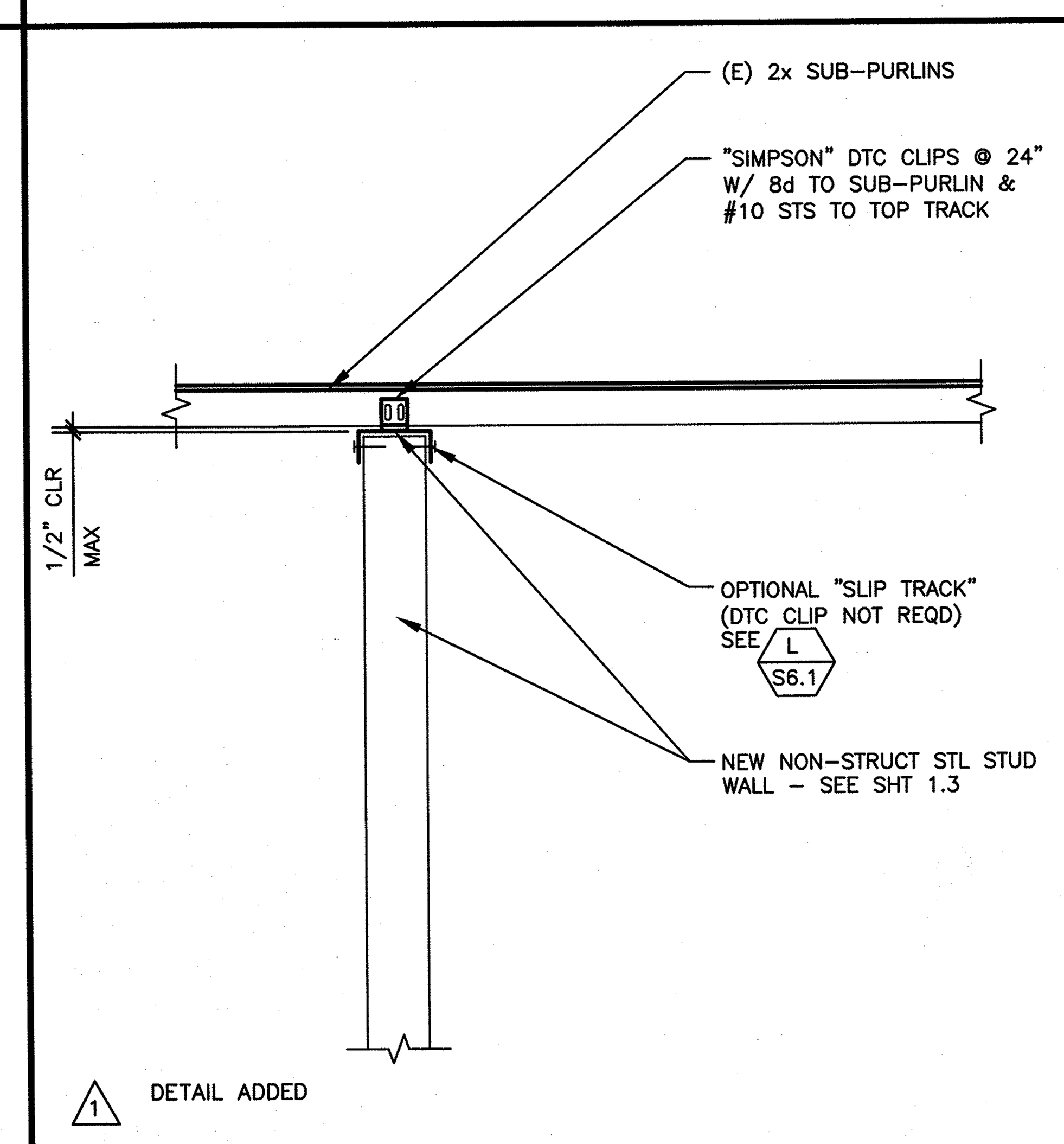
K 3/4" **SECTION AT BM CONNECTION**



L 3/4" **SECT AT NON-STRCT WALL**



M 3/4" **SECT AT NON-STRCT WALL**



N 3/4" **SECT AT NON-STRCT WALL**

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REVIEWED FOR CODE COMPLIANCE

DEC 30 2014

By________ Permit No.________

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12-12-14

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PROPOSED TENANT IMPROVEMENT FOR:

CENTER FOR HUMAN SERVICES

2000 W BRIGGSMORE, SUITE I
MODESTO, CALIFORNIA 95350

REVISIONS:	1 12-11-14 PLAN CHECK GP
DRAWN:	SG
DATE:	OCT 2014
FILE:	
JOB:	LSA 015-14
PRINT DATE:	

CONNECTION DETAILS

S6.1