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MEMORANDUM

To:Robert Stevens, CSW | ST2From:Mads Jorgensen, P.E., Dilip Trivedi, Dr.Eng, P.E.Date:February 2, 2021Subject:Beach Sand Replenishment VolumeProject:Mirada Road Bridge and Bluff StabilizationM&N Job No.:10405

1 Introduction

This memorandum provides an assessment of beach sand replenishment volumes for the proposed Mirada Road Bridge and Bluff Stabilization project (Project). At the onset of this project, a substantial quantity of rock slope protection had been emergency placed along the bluff as temporary protection. The basis of the present design assumes that this rock is removed and replaced with a significantly smaller quantity of rock slope protection at the toe of the proposed reinforced shotcrete wall (Figure 3-1). When the beach is at its summer profile, the rock slope protection will generally be buried beneath the sand. The proposed bluff stabilization design therefore significantly reduces the footprint of the structure compared to the rock placed for temporary protection.

Estimation of the beach replenishment volumes follows the Beach Sand Replenishment In-lieu Fee Worksheet and guidelines issued by the California Coastal Commission (CCC).

1.1 Site Description

Figure 1-1 shows a vicinity map of the project location (red star). The bridge is located along the coastal road between Half Moon Bay and Pacifica, approximately 1 mile south of El Granada and Pillar Point Harbor. Half Moon Bay is a crescent-shaped bay formed between Pillar Point and the bluffs downcoast from Miramar. The shoreline is exposed to waves from southwesterly and westerly directions but is somewhat sheltered from northwesterly waves in the lee of Pillar Point.

Figure 1-2 provides a location map for the project site and aerial view of the bridge.

Figure 1-3 provides an excerpt from the project drawings, MN (2020), showing the proposed bluff stabilization. The extent of the bluff stabilization along the beach fronting the bridge abutment includes a portion along the North Wall and along the South Wall. The interior portion of the bluff stabilization is intended for protection along the streambank and is not part of the sand replenishment volume assessment. The North Wall extent (W) along the beach is 123 feet and the South Wall extent is 36 feet.





Figure 1-1: Vicinity map.



Figure 1-2: Location map.

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Figure 1-3: Bluff stabilization plan, MN (2020).



2 Beach Sand Replenishment Volume

An assessment of the beach sand replenishment volume is provided in Table 2-1 following the CCC Beach Sand Replenishment In-lieu Fee Worksheet, CCC (2020).

Table 2-1 summarizes the beach replenishment volumes estimated for the North Wall and the South Wall, respectively. The parameters indicated in the table follow the notation from CCC (2020). In the first section of the table, the volume of sand (V_e) to rebuild the area of beach lost due to encroachment by the seawall is estimated. In the next section, the volume of sand (V_w) to rebuild the area of beach lost due to encroachment lost due to long-term erosion is estimated, followed by the amount of beach material (V_b) that would have been supplied to the beach if natural erosion continued. The respective volumes are then carried to the bottom of the table and added up to a total volume (V_t).

	Parameter	North Wall	South Wall
C	V _e (cy)	1,230	360
(A _e (sf)	1,230	360
	W (ft)	123	36
	E (ft)	10.0	10.0
	v (cy/sf)	1.0	1.0
	V _w (cy)	3,444	1,008
	A _w (sf)	3,444	1,008
	R (ft/yr)	0.7	0.7
	L (yr)	40	40
	V _b (cy)	1,262	369
	S (%)	43%	43%
	h _s (ft)	23.0	23.0
	h _u (ft)	0.0	0.0
	R _{cu} (ft/yr)	0.7	0.7
	R _{cs} (ft/yr)	0.0	0.0
6	V _e (cy)	1,230	360
-	V _w (cy)	3,444	1,008
	V _b (cy)	1,262	369
	V _t (cy)	5,936	1,737

Table 2-1: Sand replenishment volumes.

The calculation indicates that 5,936 cubic yard of material would be needed to replenish the beach along the North Wall, and 1,737 cubic yards along the South Wall. The total volume is 7,673 cubic yards.

The cost of the sand replenishment will depend on the material unit cost and delivery fee, which will vary depending on sourcing and contractor retained for delivery and placement of the material.

The following unit cost rates were estimated:

- Material cost for sand: \$44 per cubic yard (\$30 per ton).
- Cost including truck delivery to the site and end-dumping to the beach: \$63 per cubic yard.

Based on this unit rate the sand replenishment cost could be on the order of \$480k.

The above estimates are based on Angel Island Coarse Sand from Hanson, which is similar to the bluff material in composition. Refer to Appendix A for the gradation curve, which is included as the dark yellow line in Figure 3-7.

Values for dimensional parameters indicated in Table 2-1 were obtained from the project drawings, MN (2020). These parameters include: W, E, h_s , and h_u .

A commentary regarding the coastal engineering parameters indicated in Table 2-1 is provided in the following. The remainder of the parameters in the table were calculated based on the equations provided in CCC (2020).

3 Commentary

The following sections provide a commentary and discussion of coastal engineering parameters adopted for the beach sand replenishment volume calculation.

3.1 **Project Design Life**

The project design life (L) is 40 years.

3.2 Encroachment by Seawall

Figure 3-1 shows a representative section of the planned bluff protection from the project drawings, MN (2020). In the typical usage condition, the beach elevation is at around El. +11 feet NAVD88, based on USACE NCMP 2014 LiDAR from NOAA (2021a) as shown in Figure 3-2. In this condition, the rock slope protection fronting the reinforced shotcrete wall is buried beneath the sand. To err on the side of caution, the encroachment of the bluff protection was based on a beach face 3 feet lower at around El. +8 feet NAVD88 to account for years where the beach face doesn't build up to the full height. The estimated encroachment width is estimated to E = 10 feet, which includes the extent of exposed rock slope protection and the depth of the reinforced shotcrete wall.



Figure 3-1: Bluff protection section, MN (2020).





3.3 Closure Depth

Figure 3-3 provides an excerpt of the nautical chart for Half Moon Bay, NOAA (2021b). The project site is located at Arroyo de en Medio near Miramar.

The depth where the seabed contours start to flatten out indicates the approximate closure depth, which is estimated to 16 feet MLLW in the area of Arroyo de en Medio and Miramar per the figure. The active portion of the beach profile extends from the depth of closure up to the elevation of the beach face.

Figure 3-4 summarizes geodetic datums and tidal datums in Half Moon Bay, from NOAA (2021c). Per the figure, NAVD88 is located at nearly the same level as MLLW. The closure depth for the active portion of the beach profile is therefore estimated as: 16+11 = 27 feet. The Volume of material (v) required to replace or reestablish one foot of beach seaward of the bluff protection is therefore: $v = 27' \times 1' \times 1' = 27$ cubic feet per square foot = 1 cy/sf.



Figure 3-3: Excerpt from Navigation Chart 18682, NOAA (2021b).

ELEVATION INFORMATION

PID: VM: STATION ID: EPOCH: DATE:	HT0455 21008 9414131 1983-2001 Friday, February 16, 2018 1:45:21 PM EST					
6						
	MHHW	= 5.60 feet (1.707 meters)				
5						
	MHW	= 4.95 feet (1.509 meters)				
4	METT	- 202 fact (0.022 materia)				
3	MIL	= 5.05 feet (0.925 meters)				
	MSL	= 2.99 feet (0.911 meters)				
2	NGVD29	= 2.73 feet (0.833 meters)				
	MLW	= 1.11 feet (0.338 meters)				
1						
	MLLW	= 0.00 feet (0.000 meters)				
-1	NAVD88	= -0.04 feet-0.013 meters)				

Figure 3-4: Datum information, Half Moon Bay.

3.4 Rate of Bluff Recession

Figure 3-5 shows transects (yellow bands) developed by USGS for assessment of long-term rates of shoreline recession, USGS (2021). The data shown in the figure is a digital product developed based on the work for the National Assessment of Shoreline Change, USGS (2007), to assess rates of shoreline and coastal cliff retreat along the California Coast.

The long-term rate of recession about 1,500 feet north (and further north) of Arroyo de en Medio where the shoreline is unarmored is R = 0.2 m/yr = 0.7 feet per year.

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Figure 3-5: Long-term shoreline recession transects, USGS (2021).

3.5 Fraction of Beach Quality Material in Bluff Material

The fraction of beach quality material in the bluff material was estimated based on available project boring logs. Figure 3-6 summarizes boring locations from WRECO (2017). Boring R-17-007B was taken as a representative boring for the project location.



Figure 3-6: Boring locations from WRECO (2017).

The grain size distribution is shown in Figure 3-7. The curves pertaining to Boring R-17-007B over the height of the bluff are indicated by the following symbols and (depths): hourglass (5'), triangle (15'), and star (31.5'). The dark yellow curve is the gradation of Angel Island Coarse Sand (Appendix A).





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Figure 3-7: Grain size distribution for Boring R-17-007B, WRECO (2017).

Komar (1998) studied the relationship between beach slope and sediment size retained on beaches. He collected data for beaches in high and low wave energy environments, including Half Moon Bay, which transitions from a low energy exposure to a high energy exposure in proportion to the sheltering effected by Pillar Point. Figure 3-8 shows the relationship between median grain size of beach material relative to the slope of the beach. The slope of the beach at Arroyo de en Medio is approximately 11.8H:1V, refer to Figure 3-2. The size of material retained on the beach is around 0.45 mm as indicated by the red lines in the figure for the respective beach slope where they intersect the curve and data points for Half Moon Bay. The grain size of 0.45 mm corresponds to medium sand.



Figure 3-8: Relationship between median grain size and beach slope, Komar (1998).

The fraction of beach quality sand contained in the bluff material is estimated to be 43% based on the boring information shown in Figure 3-7, taken as the percentages by depth of the sand fraction that will remain on the beach. Figure 3-9 shows how this estimate was determined. The colored areas in the figure indicate the fraction of gravel (dark brown), sand (yellow), and silt/clay (blue) as a function of the depth. Zero depth is at the top of the bluff. The vertical dimension of 23 feet indicates the height of the bluff from the base to the top. The percentage of beach quality material was determined as the fraction of medium to coarse sand contained in the bluff material, which is the area enclosed by the dashed black line. Note, per Figure 3-8 only medium to coarse sand and gravel will remain on the beach, while material in the fine sand to silt/clay fraction will be washed out.



Figure 3-9: Composition of bluff material.

4 References

CCC (2020). Beach Sand Replenishment In-lieu Fee Worksheet. California Coastal Commission.

Komar, P.D. (1998). Beach Processes and Sedimentation. 2nd Edition. Simon & Schuster, Upper Saddle River, NJ.

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NOAA (2021a). *Coastal LiDAR.* Data Access Viewer. NOAA, National Oceanic and Atmospheric Administration, Office for Coastal Management. <u>https://coast.noaa.gov/dataviewer/#/</u>

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NOAA (2021c). *Tides & Currents.* NOAA Center for Operational Oceanographic Products and Services, National Oceanic and Atmospheric Administration, National Ocean Service. <u>https://tidesandcurrents.noaa.gov/</u>

USGS (2007). *National Assessment of Shoreline Change.* Open-File Report 2007-1133. Cheryl J. Hapke and David Reid. U.S. Department of the Interior, U.S. Geological Survey.

USGS (2021). *Coastal Change Hazards Portal.* United States Geological Survey (USGS), Science for a changing world. Version 1.1.72. <u>https://marine.usgs.gov/coastalchangehazardsportal</u>

WRECO (2017). *Geotechnical Engineering Study.* Mirada Road Retaining Wall, County of San Mateo, California. Prepared for: County of San Mateo, California. 555 County Center, Redwood City, California 94063. Prepared by: WRECO, 8331 Sierra College Boulevard, Suite 208. Roseville, California 95661. WRECO Project No. P15102 TO3. May 2017.



Appendix A

Angel Island Coarse Sand Gradation





Pier 92 480 Amador Street San Francisco, CA. 94124

12/02/2020

Lehigh Hanson

Attn:

Project Reference: General Information

This sand supplied by Hanson Aggregate is extracted from Point Knox Shoals San Francisco Bay and contains varying amounts of naturally occurring chlorides and sulfates. SMARA #'s: 91-38-0003, 91-38-0002

Procedure	Sieve/Test	Average	Unit %	Angel Island Crs Sand
	#4 (4.75mm)	95		
	#8 (2.36mm)	82	%	
	#16 (1.18mm)	60	%	
	#30 (.6mm)	33	%	
	#50 (.3mm)	13	%	
	#100 (.15mm)	1	%	
	#200 (75μm)	0.3	%	
5	FM	3.16		
CTM - 206 CTM - 206 D-4318	Specific gravity, bulk SSD Absorption Plasticity Index	2.86 1.0 +/3 % Non-Plastic		

110133-ANGEL ISLAND CRS SAND

Should you have questions regarding this aggregate material, please do not hesitate to call your Sales Representative

These data have been developed on the basis of information and tests of materials submitted to this laboratory which are assumed to be representative of the materials to be used. All test have been made in compliance with current ASTM or applicable methods of testing. ALL WARRANTIES, EXPRESSED, IMPLIED OR STATUTORY, ORAL OR WRITTEN ARE EXCLUDED EXCEPT AS SET FORTH IN HANSON AGGREGATES' STANDARD TERMS AND CONDITIONS OF SALE. NO LIABILITY ARISING OUT OF THE USE OF THESE DATA WILL BE ASSUMED BY THIS CORPORATION.

Name/Title

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